

DAVID EVANS AND ASSOCIATES INC.

DATE: March 5, 2019

TO: Jed Ireland

City of Sammamish Public Works

801 228th Avenue SE Sammamish, WA 98075

FROM: Josh Anderson, PE, PTOE and Kyle Bright, EIT

David Evans and Associates, Inc.

SUBJECT: Issaquah-Pine Lake Road Traffic Analysis

PROJECT: COSA 00000024

Issaquah-Pine Lake Road SE Improvements Ph 1, SE 32nd Way to SE Klahanie

Boulevard Project

CC: File

MEMORANDUM

1.0 INTRODUCTION

The purpose of this memorandum is to summarize the traffic operations of the Issaquah-Pine Lake Road (IPLR) SE segment between SE 32nd Way and SE Klahanie Boulevard. The memorandum informs the selection of a preferred roadway improvement alternative for the segment.

2.0 PURPOSE AND NEED FOR THE PROJECT

Issaquah-Pine Lake Road SE connects areas in south Sammamish and north and central Issaquah to areas to the north, including the City of Sammamish areas of Klahanie, Pine Lake, and Beaver Lake. To the south, Highlands Drive NE changes to Issaquah-Pine Lake Road when it crosses Issaquah Fall City Road. To the north, Issaquah-Pine Lake Road terminates at 228th Avenue SE, a major north-south arterial extending through the majority of the City.

The project is needed due to increases in traffic volumes and related congestion, delays, lack of mobility, and safety issues in the area. Nearby new development in the last decade has increased residential density as well as through traffic. The improvements as part of this project are designed to increase capacity and mobility, decrease delay and congestion, and improve motorized and nonmotorized user's safety through the project limits.

3.0 ANALYSIS MODELS

The following analysis shares similarities with the Concurrency Program, but goes into more detail and has a longer time horizon. The Concurrency Program analysis looks ahead six years consistent with the Six-Year Transportation Improvement Plan (TIP), whereas this more detailed analysis looks ahead to the forecast year of 2035. This more detailed analysis post-processes new 2018 traffic counts through the



use of the City's most current 2035 Traffic Demand Model, which is derived from the 2012 traffic model. The AM and PM peak hours were calculated from the 2018 counts and were 8:00 to 9:00 AM and 5:00 to 6:00 PM respectively. The following models were used for the operational analysis of the corridor.

3.1 SYNCHRO

The Synchro software is based upon procedures outlined in the 2000 and 2010 Highway Capacity Manuals (HCM) for intersections and arterials and explicitly evaluates traffic operations under a coordinated system of signalized intersections. It also calculates traffic arrival types, calculates right-turn-on-red capacity, and determines average and maximum expected queue lengths.

The Synchro analysis software was selected to perform the intersection analysis as it can provide the delay and LOS output of an HCM analysis and consider the systematic interaction of the intersections regarding queuing and delays. HCM 2010 methodology was used for signalized and unsignalized intersections where two-stage left turns are not present. HCM 2000 procedures were used at unsignalized intersections were two stage left-turns are present. Synchro models for existing (2018), no build, and build were created to analyze intersection level delay and LOS to compare to City operational standards for AM and PM time periods. The Synchro model was comprised of the IPLR study area with additional intersections at SE 47th Way and 230th Ln SE.

3.2 SIDRA

Sidra was used to analyze roundabouts for intersection level delay and LOS to compare to City operational standards. For existing, no build, and build conditions, the roundabout at SE 32nd Way was analyzed. A roundabout was also analyzed at Klahanie Blvd as a build option.

3.3 VISSIM

Microsimulation models were created in VISSIM for existing, no build, and build conditions to measure and quantify driver experience. Before the analysis could begin, the VISSIM models were calibrated for local conditions. For the calibration, traffic volumes, lane configurations, lane utilization, signal timing, and driver behavior were adjusted from the default settings. This was done to allow the simulation model to replicate local volumes, and queuing and travel time observations. The model was then run for 90 minutes (7:30 to 9:00) in the AM and 75 minutes (4:45 to 6:00) in the PM peak periods. The AM model is longer than the PM model to capture the impact of the Pine Lake Middle School speed zone that occurs between 7:45 and 8:15 AM as well as the period with the highest volume.

The simulation was conducted with a volume peaking profile that was obtained from the turning movement counts to replicate the peaking traffic patterns that were observed in the field. Instead of

using one set of volumes for the peak hour and assuming the vehicles arrive evenly spread throughout the hour, a separate volume set was created for each 15-minute interval. This allows for a more detailed simulation of short events such as school drop-offs or pick-ups. Turning speeds and headway factors were adjusted and the models were re-simulated and, once again, compared to the field observations. This process was repeated until the model queue lengths were visually comparable to the actual field observations. Once this visual level of calibration was gained, volume throughputs and field collected travel times were summarized from the simulations and compared to actual field collected data.

GEH Statistic:

$$GEH = \sqrt{\frac{2(v-C)^2}{v-C}}$$

v = Model Volume C = Counted Volume



To confirm volume calibration, the industry uses a statistic called the GEH statistic. The formula for GEH is shown in the box to the right. The goal is to calculate a GEH statistic for each turning movement in the model and have 85 percent of the GEH statistics below 5. If the GEH is between 5 and 10, additional investigation should be conducted, and if the value is above 10, there is a high probability that either the counted volume, or the modeled volume is inaccurate.

The existing models were calibrated to where the GEH statistic for 100 percent of the turning movement volumes was below 2.5. To confirm travel time calibration, the field collected travel times were compared to the modeled travel times. Travel time calibration is not as complicated as volume calibration. The industry standard for travel time calibration is to have the modeled travel times within ten percent of the observed travel times. The morning and evening model travel times were within six percent of the observed travel times.

4.0 EXISTING CONDITIONS

This section summarizes the existing (2018) conditions on IPLR. Volumes used in the analysis of the existing conditions were collected at the onset of the project in 2018. Synchro was used to analyze the signalized and unsignalized intersections. Sidra was used to analyze the roundabout at SE 32nd Way. VISSIM was used to identify queuing deficiencies.

4.1 Traffic Volumes

The turning movement traffic counts were collected at study area intersections on May 17th of 2018 between 7:00 and 9:00 AM and 4:00 and 6:00PM. Directional tube counts were collected for the 48 hours of May 16th and 17th of 2018. The traffic counts are included as **Appendix A.**

The turning movement volumes for the AM and PM peak hours are shown in **Figure 1**. The morning peak hour occurred between 8:00 and 9:00 AM while the afternoon peak hour occurred between 5:00 and 6:00 PM.

4.2 Segment and Corridor Operations

In early December of 2018, the Sammamish City Council passed an emergency ordinance adopting a new segment and corridor volume to capacity standard. As of the writing of this memo, Council has not yet permanently codified the new segment and corridor capacity standards; however, it is anticipated that they will in the near future. The IPLR project is covered by segments 30 and 31 (SE 32nd Way-SE Klahanie Blvd). Segments 32 and 33 (SE Klahanie Blvd-SE 48th St) are south of the project study area and make up a future phase of the IPLR widening. The second phase is envisioned as a five-lane corridor-widening project from Klahanie Blvd to SE 48th St.

Table 1 shows the segment and corridor operations when compared to the new City standards (this table was taken from the City's recently passed emergency ordinance. The table shows volumes for year 2016 for segments 32 and 33, and volumes from the new 2018 counts for segments 30 and 31. All of the segments and corridors are currently meeting the City's newly adopted standards.



Table 1: Segment and Corridor Operations for year 2016/2018 (Without IPLR Phase 1 or 2)

				:	2016/	2018	HCN	1 Modi	fied I	Meth	odol	ogy					
							Cha	racteristi	cs				2035 HCM			AM	PM
	Segment*		AM Volume	PM Volume	Speed limit (mph)	# Lanes	LT Lane	Median	RT Lane	ITS	FYA	HCM Category	Modified Capcity	AM V/C	PM V/C	Corrid Segme	
	Issaguah-Pine Lake Road Corrido	EB/S	В											0.98	0.86	Pass	Pass
	issaquali-Pille Lake Road Collido	WB/	/NB											0.58	1.09	Pass	Pass
30	lssaquah-Pine Lk Rd -	EB	510	829	35	2	1					1	881	0.58	0.94	Pass	Pass
30	228 th Ave SE to SE 32 nd Way ¹	WB	611	678	3		1					1	001	0.69	0.77	Pass	Pass
31	Issaquah-Pine Lk Rd -	NB	534	782	35	2	1					1	881	0.61	0.89	Pass	Pass
31	SE 32nd Way to SE Klahanie	SB	618	745	33		1					1	001	0.70	0.85	Pass	Pass
32	Issaquah-Pine Lk Rd -	NB	391	990	35	2	1					1	881	0.44	1.12	Pass	Pass
32	SE Klahanie to SE 46th St ²	SB	979	742	33		1					1	001	1.11	0.84	Pass	Pass
33	Issaquah-Pine Lk Rd -	NB	444	1,207	35	2	1					1	881	0.50	1.37	Pass	Pass
	SE 46th St to SE 48th St ²	SB	1,078	717	35		1					1	881	1.22	0.81	Pass	Pass

Notes

Corridor V/C ratios are volume weighted.

4.3 Traffic Operations

A summary of the existing lane configurations and traffic control devices at study area intersections is shown in **Figure 2**. The existing traffic operations analysis is presented below.

Delay and LOS

The Synchro and Sidra analysis produced results for critical movement, LOS and delay. As can be seen in Table 2, below, all intersections are currently operating within the City's operational standard of LOS D. However, field observations showed deficiencies at the SE 32nd Way roundabout were not captured by this model, which led to the more detailed simulation analysis below.

Table 2: Existing Intersection Operations

Intersection	Critical	8-9	AM	5-6	РМ
Intersection	Movement	LOS	Delay	LOS	Delay
1: ILPR @ SE 32nd Way					
(Roundabout)	Overall	Α	5.5	Α	5.7
2: ILPR @ 234th Ave SE (2-way stop)	EB Left	С	15.4	С	16.8
3: ILPR @ SE 37th Pl (2-way stop)	WB Left	С	16.0	С	17.3
4: ILPR @ SE 40th PI (2-way Stop)	WB Left	С	15.0	С	16.5
5: ILPR @ SE 42nd St (Signal)	Overall	Α	6.1	Α	6.4
6: ILPR @ Klahanie Blvd (Signal)	Overall	В	19.9	В	15.9

Source: DEA created and maintained Synchro and Sidra models (2018)

Travel Time and Queuing

It is common for the IPLR corridor to experience stop and go operations approaching the 32nd Way roundabout during various hours of the day. However, the deficiencies don't show up in the table above. This is due to operational deficiencies that are beyond the ability of the HCM compatible Synchro and Sidra programs to replicate. To capture these operational deficiencies, a VISSIM simulation model was used to analyze additional operational parameters to accurately reflect driver behaviors.

 $^{^{\}rm 1}$ This segment transitions from a wider cross-section to two lanes, the narrower section was used.

² Volumes for these segments are consistent with the 2016 Concurrency update as 2018 volumes were not collected for these segments.



Travel times through the corridor were calculated to form a baseline from which to compare the future scenarios. In the morning, it currently takes as much as three minutes to travel south from the signal at 230th Lane SE, to the signal at Klahanie Boulevard. In the evening, it also currently takes as much as three minutes to travel north from the signal at Klahanie Boulevard to the signal at 230th Lane SE.

As such, the simulation model for the entire study area was reviewed and notable queuing was only observed at the 32nd Way roundabout. As the modeled queuing was consistent with the queuing observed in the field investigations, the existing model was deemed calibrated. The queuing observations are summarized in Table 3.

Table 3: Existing queue lengths approaching the IPLR/32nd Way intersection

	7:30 - 9:00 AM	4:45 - 6:00 PM
Westbound	25	8
Northbound	12	30
Southbound	15	12

NOTE: Queue lengths shown in number of vehicles.

5.0 NO-BUILD CONDITIONS

This section summarizes the operations of IPLR using 2035 volumes and the existing road geometry. In essence, summarizing what traffic conditions would be expected to look like if no improvements to the corridor were constructed.

5.1 Traffic Volumes

To calculate the future year 2035 demands, the City's adopted year 2035 travel demand model was used. The growth between the base year model volumes (2012) and the future year model volumes (2035) was calculated. The model growth was added to the existing 2018 counts, but because the counts to be grown were collected in 2018, only 17 years (2018 to 2035) of the 23 years (2012 to 2035) of model volume growth was used or approximately 74%. The resulting year 2035 projected volumes are shown in **Figure 3.**

5.2 Segment and Corridor Operations

Table 4 shows the segment and corridor operations when compared to the new City standards. The project level traffic demands are shown for segments 30 and 31 while the City's currently adopted 2035 VISUM model (paired with the 2012 base model) was used to estimate the 2035 traffic demands on segments 32 and 33. The northern segments (IPLR phase 1) are expected to meet the City's newly adopted standards while the southern segments (IPLR phase 2) are not expected to meet the City's newly adopted standards. In the morning and evening peak periods, the southbound and northbound corridors fail, respectively, as the overall corridor v/c ratios exceed the 1.1 standard.



Table 4: Segment and Corridor Operations for year 2035 (Without IPLR Phase 1 or 2)

					203	35 H	CM N	lodifie	d Me	thod	ology	,					
							Cha	aracteristi	cs							AM	PM
	Segment*		AM PM Volume		Speed # LT Median RT Lane ITS FYA		FYA	HCM Category	2035 HCM Modified Capcity	AM V/C	PM V/C		or ≤1.1 :nt ≤1.4				
	Issaquah-Pine Lake Road Corrido	EB/S	SB											1.12	1.01	Fail	Pass
	issaquali-Fille Lake Koau Collido	WB,	/NB											0.80	1.41	Pass	Fail
30	Issaquah-Pine Lk Rd -	EB	705	1,055	35	2	1					1	881	0.80	1.20	Pass	Pass
30	228 th Ave SE to SE 32 nd Way ¹	WB	750	940	33		1					1	001	0.85	1.07	Pass	Pass
31	Issaquah-Pine Lk Rd -	NB	740	1,035	35	2	1					1	881	0.84	1.17	Pass	Pass
31	SE 32 nd Way to SE Klahanie	SB	760	820	33		1					1	001	0.86	0.93	Pass	Pass
32	Issaquah-Pine Lk Rd -	NB	690	1,370	35	2	1					1	881	0.78	1.56	Pass	Fail
32	SE Klahanie to SE 46th St	SB	1,110	790	33		1					1	881	1.26	0.90	Pass	Pass
33	Issaquah-Pine Lk Rd -	NB	620	1,465	35	2	1					1	881	0.70	1.66	Pass	Fail
33	SE 46th St to SE 48th St	SB	1,195	825	35		1					1	881	1.36	0.94	Pass	Pass

Notes

Corridor V/C ratios are volume weighted.

5.3 Traffic Operations

Delay and LOS

To be consistent with the City's operational standards, an HCM compliant analysis was conducted. The analysis produced operational results for critical movement, LOS, and delay. As can be seen in Table 5, delay increased for all intersections compared to the existing conditions. In the AM, the LOS at Klahanie Blvd went from LOS B to LOS E, which is beyond the City operational standards. In the PM, all intersections operate within the City operational standards.

Table 5: 2035 No-Build Intersection Operations

Intersection	Critical	8-9	АМ	5-6	РМ
Intersection	Movement	LOS	Delay	LOS	Delay
1: ILPR @ SE 32nd Way					
(Roundabout)	Overall	В	10.9	В	14.3
2: ILPR @ 234th Ave SE (2-way stop)	EB Left	С	21.1	D	26.7
3: ILPR @ SE 37th Pl (2-way stop)	WB Left	С	20.0	С	24.4
4: ILPR @ SE 40th Pl (2-way stop)	WB Left	С	20.1	С	23.0
5: ILPR @ SE 42nd St (Signal)	Overall	Α	7.6	Α	8.3
6: ILPR @ Klahanie Blvd (Signal)	Overall	Е	55.1	С	33.9

Source: DEA created and maintained Synchro and Sidra models (2018)

Travel Time and Queuing

In general, the travel times through the study area increased from the existing 2018 conditions to the future 2035 conditions. In the morning, the southbound travel times are projected to increase by approximately 25%. In the evening, the northbound travel times are projected to increase by approximately 60%.

¹ This segment transitions from a wider cross-section to two lanes, the narrower section was used.



The increased travel times are due to the declining operations of the intersections of Klahanie Boulevard and 32nd Way. After reviewing the simulation, the 32nd Way roundabout was again the only area with notable queuing, as shown in Table 6.

Table 6: Background queue lengths approaching the IPLR/32nd Way intersection

	7:30 - 9:00 AM	4:45 – 6:00 PM
Westbound	30	10
Northbound	28	40
Southbound	45	42

NOTE: Queue lengths shown in number of vehicles.

6.0 BUILD CONDITIONS

The build scenarios seek to upgrade the corridor to current design standards and provide bicycle and pedestrian amenities and better transit access through the corridor while also providing mitigation for deficiencies that were observed in the no-build scenario.

The no-build results for Klahanie Blvd showed that the intersection would operate at LOS E, which is beyond the City's operational standards, and the simulation identified excessive queuing approaching the SE 32nd Way roundabout.

To mitigate the deficiency at the Klahanie Blvd signalized intersection, two improvements were identified. The first improvement updates signal timing and operational inefficiencies to slightly improve operations within the City's standards. The second improvement converts the signalized intersection to a roundabout with two southbound through lanes and dedicated northbound and westbound right-turn slip lanes to more fully address the projected failure.

Two improvements were also identified to mitigate the deficiency at the SE 32nd Way roundabout. The first improvement is to increase the capacity of the roundabout with a northbound right turn slip lane and a dedicated southbound left turn lane while also updating the overall design to improve vehicular flow through the roundabout. The second improvement would convert the roundabout to a signalized intersection.

While the intersection of SE 37th Place meets the City's operational standards, there are a lack of protected pedestrian crossings of IPLR, limiting access to transit. Signal warrant analyses were conducted at the unsignalized intersections of SE 40th Place, SE 37th Place, and 234th Avenue SE. SE 37th Place was the only intersection to meet signal warrants and is located roughly midway between the formalized pedestrian crossings at SE 42nd Street and SE 32nd Way. As such, a signal is proposed for the SE 37th Place intersection. Signal warrant summary sheets can be found in **Appendix B**.

Figure 4 shows the proposed intersection lane configurations as well as traffic control devices.

6.1 Segment and Corridor Operations

In early December of 2018, the Sammamish City Council adopted an emergency ordinance which established new segment and corridor volume to capacity standards. The IPLR project is covered by segments 30 and 31. Segments 32 and 33 are south of the project study area and make up a future phase of the IPLR widening. The second phase is envisioned as a five-lane corridor-widening project from Klahanie Blvd to Issaquah Fall City Road.



If the future phase of the IPLR project is not completed, the corridor will not meet the City's standards, as can be seen in Table 7.

Table 7: Segment and Corridor Operations for year 2035 (Without IPLR Phase 2)

					203	35 HC	M M	lodifie	d Me	thod	ology	,					
							Cha	aracteristi	cs							AM	PM
	Segment*		AM Volume	PM Volume	Speed limit (mph)	# Lanes	LT Lane	Median	RT Lane	ITS	FYA	HCM Category	2035 HCM Modified Capcity	AM PM V/C V/C		Corridor ≤1.1 Segment ≤1.4	
	Issaquah-Pine Lake Road Corrido	EB/S	В											1.09	0.94	Pass	Pass
	issaquali-Fille Lake Road Collido	WB,	/NB											0.75	1.36	Pass	Fail
30	Issaquah-Pine Lk Rd -	EB	705	1,055	35	2	1	1		1	1	1	987	0.71	1.07	Pass	Pass
30	228 th Ave SE to SE 32 nd Way ¹	WB	750	940	33		1	1		1	1	1	967	0.76	0.95	Pass	Pass
31	Issaquah-Pine Lk Rd -	NB	740	1,035	35	2	1	1		1	1	1	987	0.75	1.05	Pass	Pass
31	SE 32 nd Way to SE Klahanie	SB	760	820	33		1	1		1	1	1	967	0.77	0.83	Pass	Pass
32	Issaquah-Pine Lk Rd -	NB	690	1,370	35	2	1					1	881	0.78	1.56	Pass	Fail
32	SE Klahanie to SE 46th St	SB	1,110	790	33		1					1	981	1.26	0.90	Pass	Pass
33	Issaquah-Pine Lk Rd -	NB	620	1,465	35	2	1					1	881	0.70	1.66	Pass	Fail
33	SE 46th St to SE 48th St	SB	1,195	825	33		1					1	001	1.36	0.94	Pass	Pass

Notes

Corridor V/C ratios are volume weighted.

If the widening of the second phase of IPLR is constructed (assumed to occur before 2035), all segments and the overall corridor meet the newly adopted City standards as seen in Table 8.

Table 8: Segment and Corridor Operations for year 2035 (With IPLR Phase 2)

					203	35 HC	CM N	lodifie	d Me	thod	ology	,					
							Cha	aracteristi	cs							AM	PM
	Segment*		AM Volume	PM Volume	Speed limit (mph)	# Lanes	LT Lane	Median	RT Lane	ITS	FYA	HCM Category	2035 HCM Modified Capcity	AM V/C	PM V/C	Corrid Segme	
	Issaquah-Pine Lake Road Corrido	EB/S	ВВ											0.66	0.72	Pass	Pass
	issaquali-Fille Lake Road Collido	WB,	/NB											0.56	0.85	Pass	Pass
30	Issaquah-Pine Lk Rd -	EB	705	1,055	35	2	1	1		1	1	1	987	0.71	1.07	Pass	Pass
30	228 th Ave SE to SE 32 nd Way ¹	WB	750	940	33		1	1		1	1	1	967	0.76	0.95	Pass	Pass
31	Issaquah-Pine Lk Rd -	NB	740	1,035	35	2	1	1		1	1	1	987	0.75	1.05	Pass	Pass
31	SE 32 nd Way to SE Klahanie	SB	760	820	33		1	1		1	1	1	367	0.77	0.83	Pass	Pass
32	Issaquah-Pine Lk Rd -	NB	690	1,370	35	4	1	1		1	1	2	1,896	0.36	0.72	Pass	Pass
32	SE Klahanie to SE 46th St	SB	1,110	790	35	4	1	1		1	1	2	1,896	0.59	0.42	Pass	Pass
33	Issaquah-Pine Lk Rd -	NB	620	1,465	35	4	1	1		1	1	2	1,896	0.33	0.77	Pass	Pass
33	SE 46th St to SE 48th St		1,195	825	33	4	1	1		1	1	2	1,890	0.63	0.44	Pass	Pass

Notes

Corridor V/C ratios are volume weighted.

6.2 Traffic Volumes

The traffic volumes in the build conditions are held consistent with those of the no-build conditions.

 $^{^{\}rm 1}$ This segment transitions from a wider cross-section to two lanes, the narrower section was used.

 $^{^{\}rm 1}$ This segment transitions from a wider cross-section to two lanes, the narrower section was used.



6.3 Traffic Operations

The build operations results were compared to the no-build results to confirm that the proposed mitigations address the deficiencies observed in the no-build models.

Delay and LOS

The Synchro and Sidra analysis produced operational results for critical movement, LOS and delay. The table below shows the results of the build scenario.

Replacing the roundabout at SE 32nd Way with a traffic signal would increase delay by about ten seconds on average and the intersection LOS would increase from A to B. While there would be increased delay according the HCM intersection equations, the queuing is expected to be much improved with either the new roundabout design or the new signalized design.

The intersection at Klahanie Blvd is expected to operate at LOS A as a roundabout, and LOS D and C in the AM and PM peak hours, respectively. With the proposed mitigations, all intersections would meet the City's operational standards.

The full operations are shown in Table 9.

Table 9: 2035 Build Intersection Operations

Intersection	Critical	8-9	AM	5-6	РМ
Intersection	Movement	LOS	Delay	LOS	Delay
1: ILPR @ SE 32nd Way (Roundabout)	Overall	Α	8.6	Α	6.3
1: ILPR @ SE 32nd Way (Signal)	Overall	В	16.6	В	15.6
2: ILPR @ 234th Ave SE (2-way stop)	EB Left	С	21.5	D	28.5
3: ILPR @ SE 37th Pl (Signal)	Overall	Α	9.9	В	12.1
4: ILPR @ SE 40th Pl (2-way stop)	WB Left	С	20.1	С	23.0
5: ILPR @ SE 42nd St (Signal)	Overall	Α	7.6	Α	8.3
6: ILPR @ Klahanie Blvd (Signal)	Overall	D	54.7	С	33.9
6: ILPR @ Klahanie Blvd (Roundabout)	Overall	Α	9.2	Α	7.7

Source: DEA created and maintained Synchro and Sidra models (2018)

Travel Time and Queuing

In general, the travel times through the study area were much improved when compared to the no-build 2035 conditions. Simulation models were created for a build option with signalized intersections at Klahanie and 32nd, and with roundabout intersections at Klahanie and 32nd. In the morning, the signalized and roundabout options operate similarly with southbound travel times decreasing by between 10 and 40%. In the evening, the northbound travel times decrease by between 5% and 45%.

Either of the build options would be expected to drastically improve travel times in the near future and allow travel times in the year 2035 to be similar to existing levels.

Queuing deficiencies approaching the 32nd Way roundabout are shown in Table 10. For comparative purposes, the background (2035 volumes and existing geometry) queue lengths are also shown.



Table 10: Background queue lengths approaching the IPLR/32nd Way intersection

		7:30 - 9:00 AM			4:45 - 6:00 PM	
	2035	2035 Build	2035 Build	2035	2035 Build	2035 Build
	Background	Roundabout	Signal	Background	Roundabout	Signal
Westbound	30	28	7	10	10	6
Northbound	28	8	20	40	34	40
Southbound	45	11	2	42	10	5

NOTE: Queue lengths shown in number of vehicles.

For the southbound and westbound directions, the signal results in the most improved queuing, while in the northbound direction, the roundabout results in the most improved queuing.

7.0 ROUNDABOUTS – BENEFITS AND DRAWBACKS

7.1 Environmental Impact

In some cases, roundabouts can allow for a reduction in greenhouse gas emissions by reducing or all together eliminating vehicle stops and starts. As stopping and starting reduces, the time spent idling is decreased which translates into less fuel consumption and fewer emissions.

7.2 Vehicular Safety

Many studies have shown that roundabouts, when designed properly, are safer intersection treatments than standard signalized intersections. Roundabouts result in fewer conflict points and the ones that do exist typically result in less severe crashes, typically sideswipe and rear-end instead of angle or head on crashes. A Minnesota DOT study concluded that single lane roundabouts have been shown to reduce fatal and serious injury crashes by over 80 percent and reduce overall crashes by over 30 percent.

7.3 Yield Confusion

Roundabouts with multiple circulating lanes don't have the same safety record as the single lane roundabouts. Injury crashes have been shown to decrease for single lane roundabouts, while non-injury (property damage only) crashes often increase due to the confusion of yielding upon entry or exit of the roundabout for multiple lanes.

7.4 Pedestrian Safety

Pedestrian crossings are not protected at roundabouts. The pedestrians are relying on the drivers to yield to them as they attempt to cross the approaches of the roundabout. Individuals with vision impairments have difficulty determining when traffic is yielding to them. As bicyclists are suggested to move onto the sidewalk and traverse the roundabouts using the striped crosswalks, the safety implications for pedestrians also apply to bicyclists.

7.5 Emergency Vehicle Priority

With a roundabout, there is no way to give priority to an approaching emergency vehicle. The emergency vehicles priority relies on other drivers noticing the emergency vehicle, getting out of the way, and staying out of the way.



7.6 Unbalanced Traffic Flows

Unbalanced flows on the approaches to a roundabout can lead to drivers ignoring the yielding rules. If the majority of the flow is in the north/south directions and they usually don't have to yield for circulating vehicles, then drivers get in the pattern and quickly learn to not stop, even in the case of a circulating vehicle.

7.7 Non Peak Hour Vehicle Delays

During times of lower traffic volumes (mid-day, nights, weekends) the roundabout would be expected to have less delay than a comparable signalized intersection as vehicles from all four approaches are capable of entering the roundabout at the same time and traversing the intersection. In a signal, each approach must wait its turn for the green light.

7.8 Right-of-way Needs

When properly designed, roundabouts require more right-of-way than a signal. The curves on the approaches, and the correct sizing of the center island and approach islands require more land than a typical signalized intersection.

7.9 Traffic Calming

By physically slowing vehicles as they approach the roundabout, they can provide a good transition between higher and lower speed areas. The school speed zone at the north end of the IPLR corridor is a great example of the transition between a higher speed corridor and a lower speed section of the corridor.

7.10 Inclement Weather

Vertical grades when approaching roundabouts can be problematic in areas that are prone to snow and ice. If an approach to a roundabout is going downhill, the curves of the roundabout approach can compound the driver's inability to stop.

7.11 Landscaping

Roundabouts in Sammamish are typically landscaped and require ongoing maintenance.

8.0 CONCLUSIONS AND RECOMMENDATIONS

The traffic analysis has shown that the IPLR corridor between Klahanie Boulevard and SE 32nd Way should be widened to a three-lane cross section to accommodate turning maneuvers and multi-modal transportation needs. In addition, it is assumed that all signalized intersections will be upgraded to include Intelligent Transportation System operations, and Flashing Yellow Arrows, where appropriate.

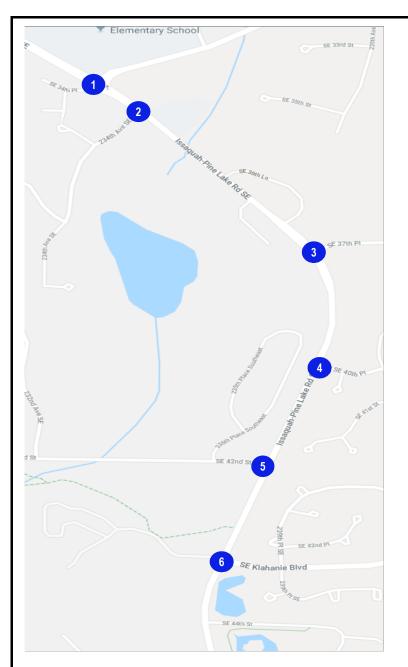
The intersection of IPLR at SE 32nd Way can be improved with either an updated roundabout or signalized intersection design. However, it is recommended that the intersection be converted to a signal due to the lack of available right-of-way and proximity to the elementary school. Lack of available right-of-way prevents optimization of the roundabout, and the circulation patterns of Sunny Hills Elementary and pedestrian crossings operate more efficiently with a signalized intersection.

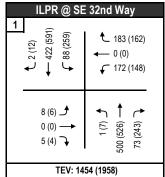


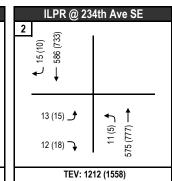
The intersection of IPLR at SE 37th Place is an ideal location for the installation of a traffic signal to facilitate safe vehicular turning movements onto and off IPLR. The signal will also provide for a protected pedestrian crossing of IPLR.

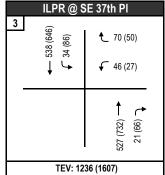
The temporary signal at the intersection of IPLR at SE 42nd Street should be replaced with a permanent signal and the eastbound approach should be widened to provide for a dedicated left-turn lane and a dedicated right-turn lane.

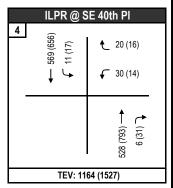
The intersection of IPLR at Klahanie Boulevard could be improved by the installation of either an updated signal or a roundabout. While both designs meet the applicable City standards, the roundabout design is expected to function more efficiently than the signal. However, both options should remain as further discussions progress with the Issaquah School District regarding access to a proposed school to the west of IPLR and south of Klahanie Boulevard.

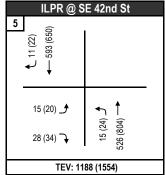


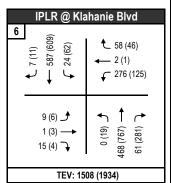














Legend

(###) Peak |
TEV: ### (###) Total

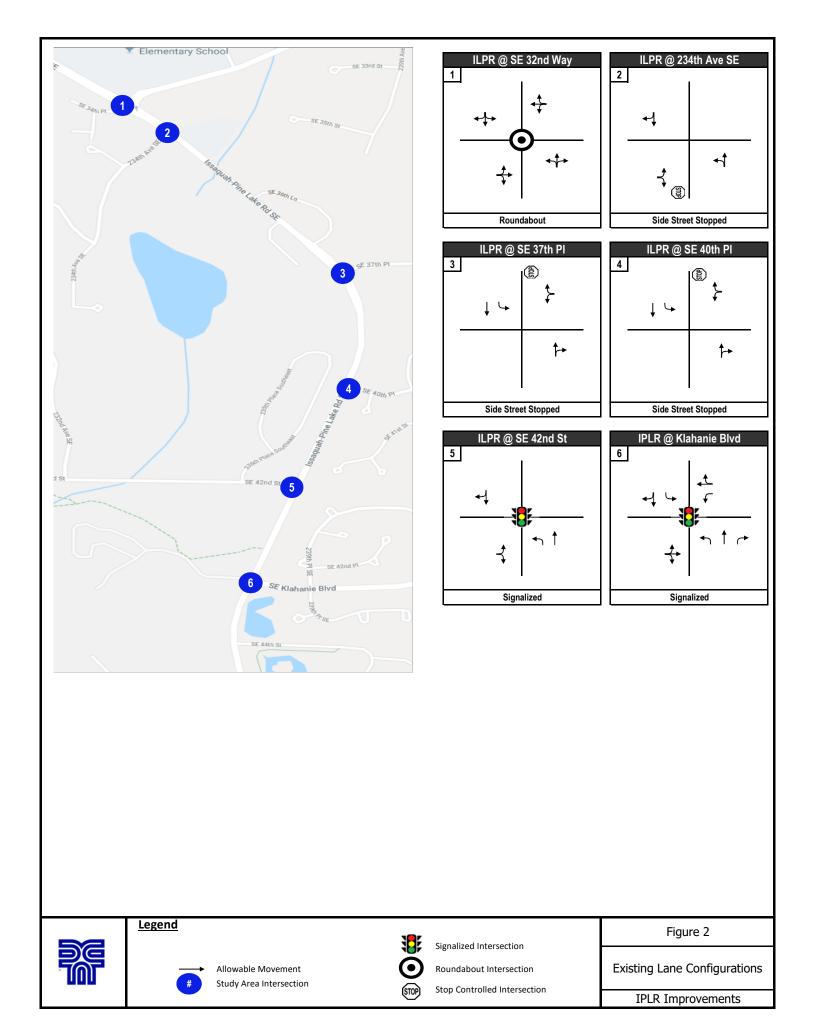
Peak Hour Volume: AM (PM) Total Entering Volume: AM (PM)

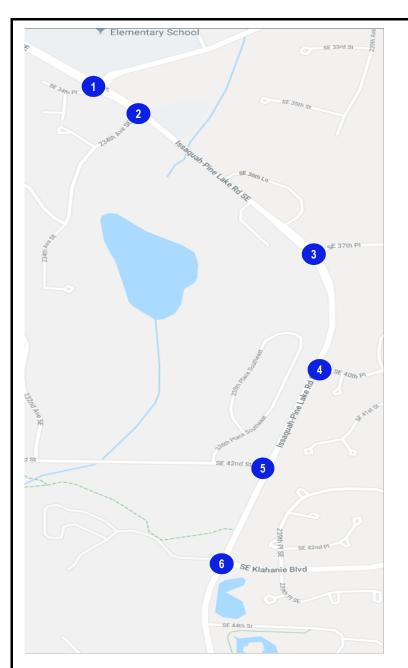


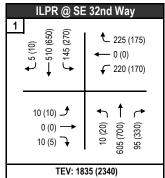
Allowable Movement Study Area Intersection Figure 1

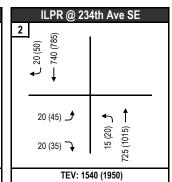
Existing Turning Movement Volumes

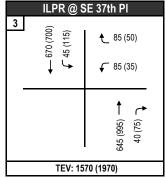
IPLR Improvements

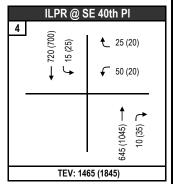


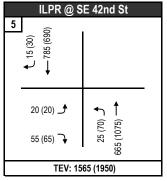


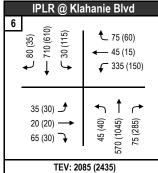














Legend

(###) TEV: ### (###)

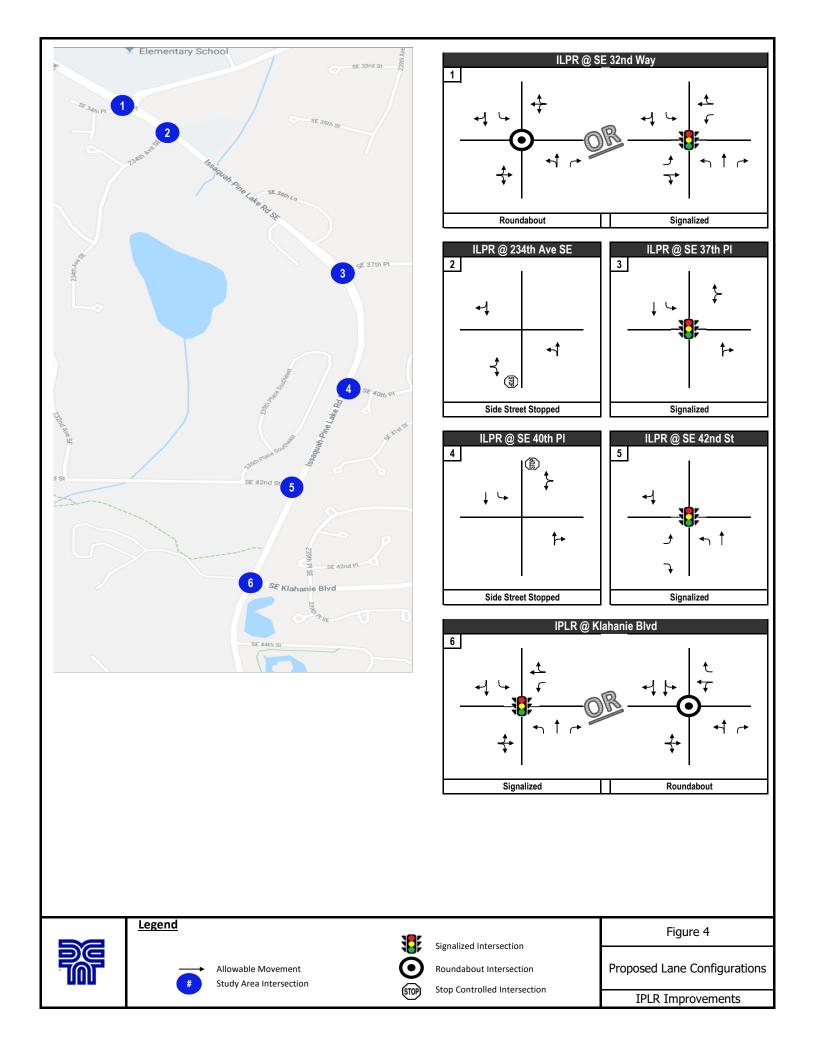
Peak Hour Volume: AM (PM)
Total Entering Volume: AM (PM)



Allowable Movement Study Area Intersection Figure 3

Future 2035 Turning Movement Volumes

IPLR Improvements



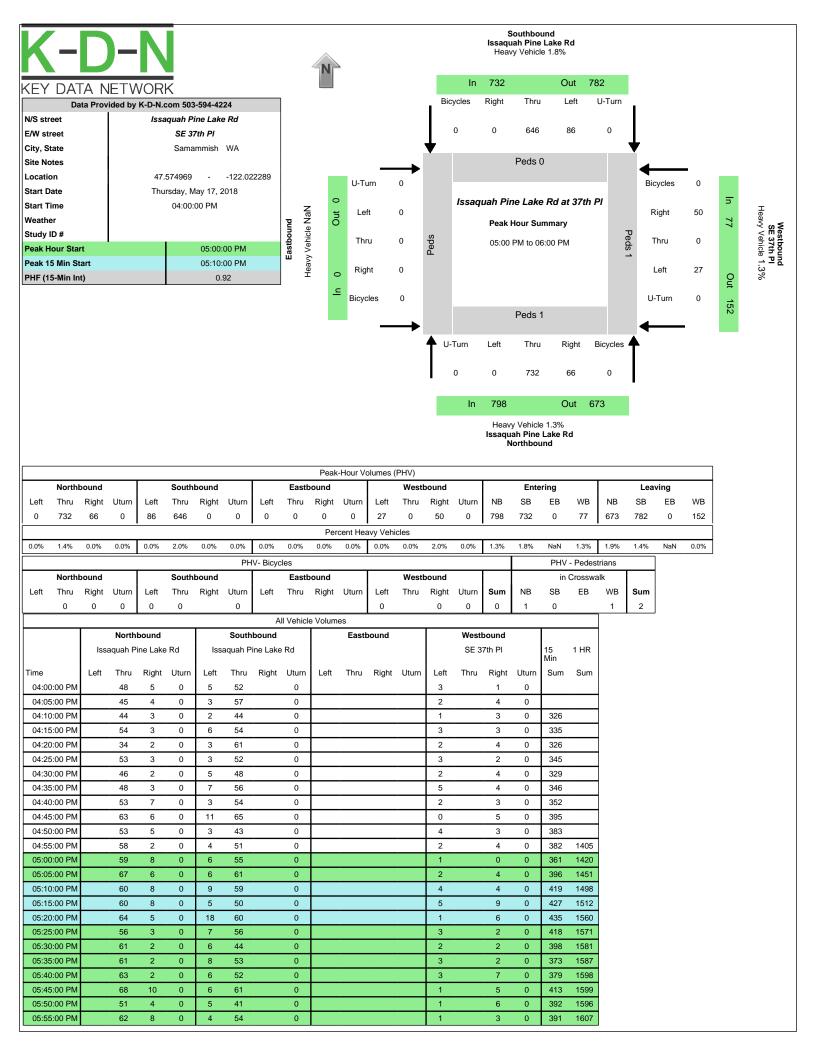


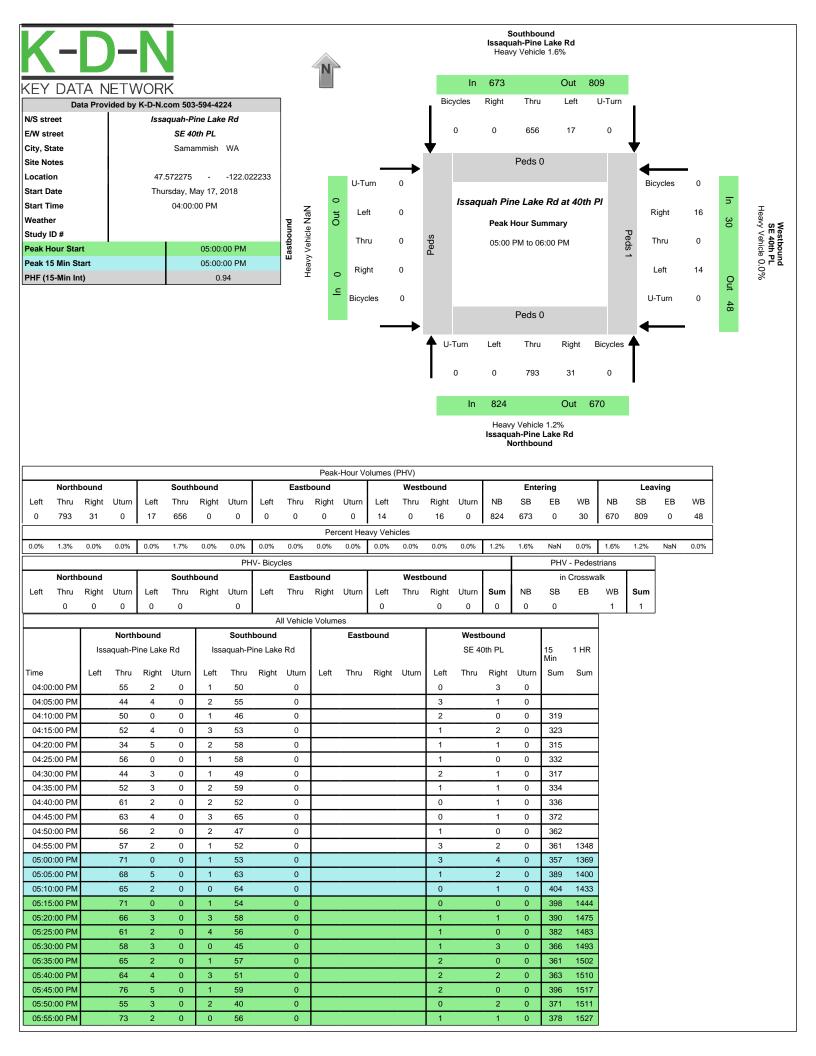
APPENDICES

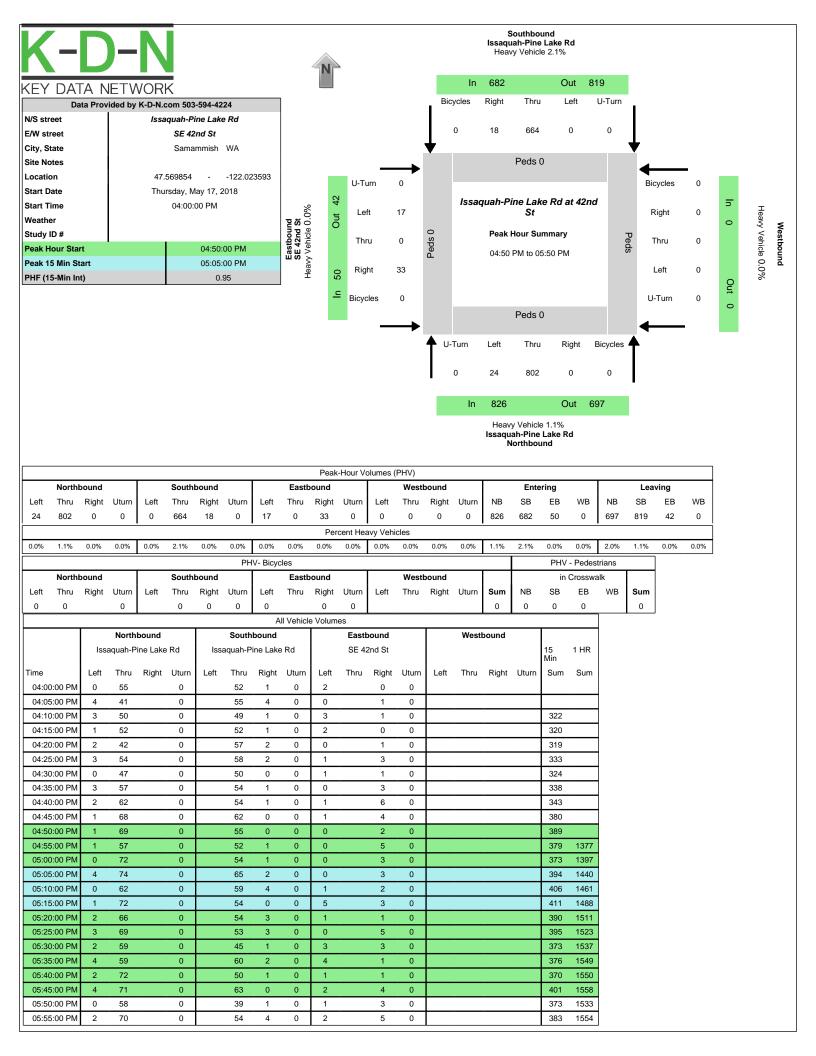


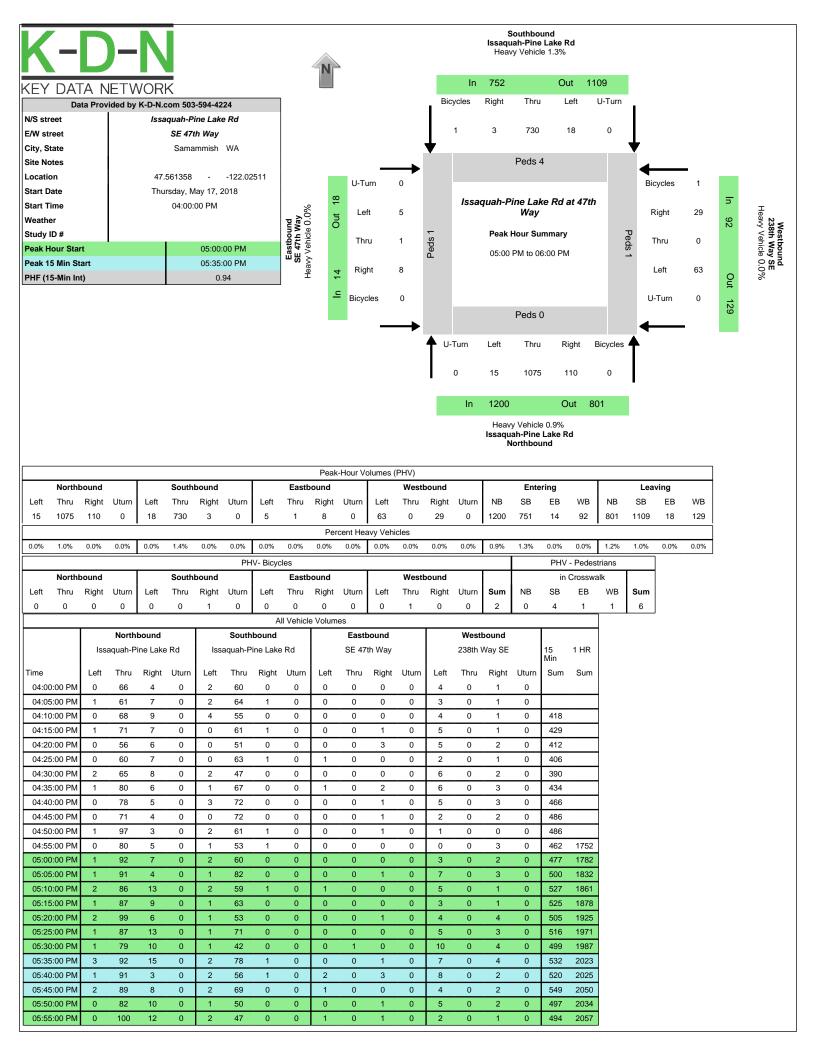
APPENDIX A:

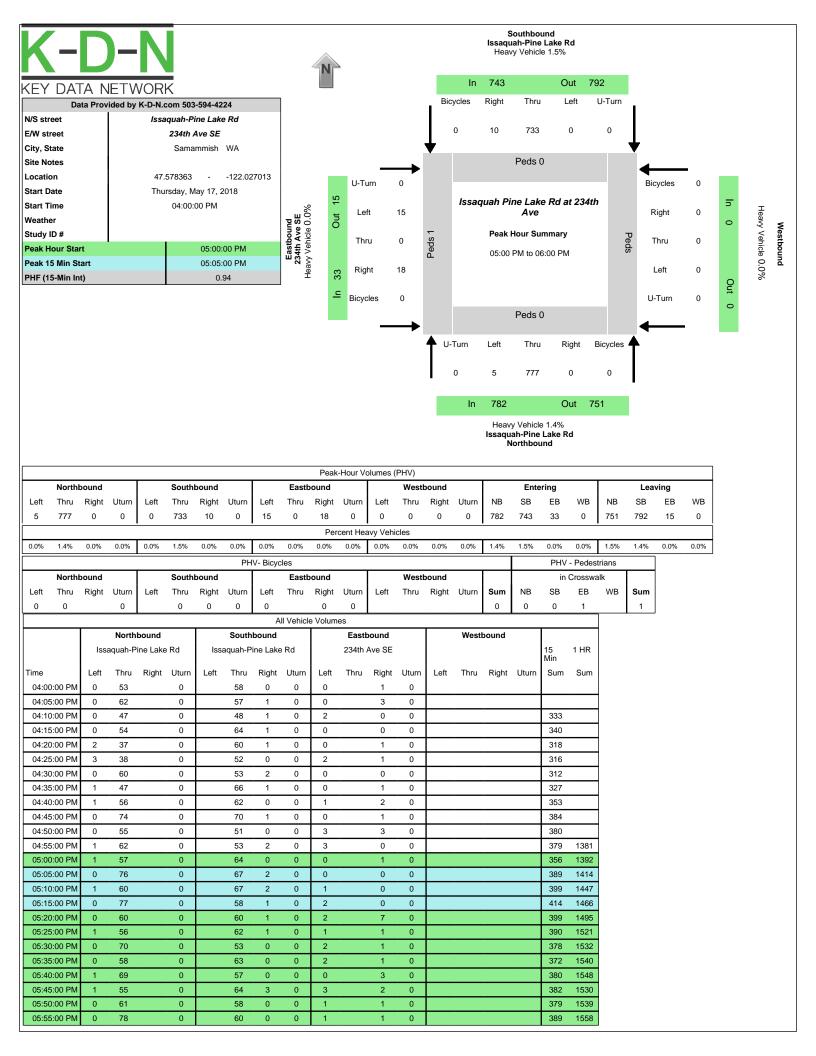
Traffic Counts

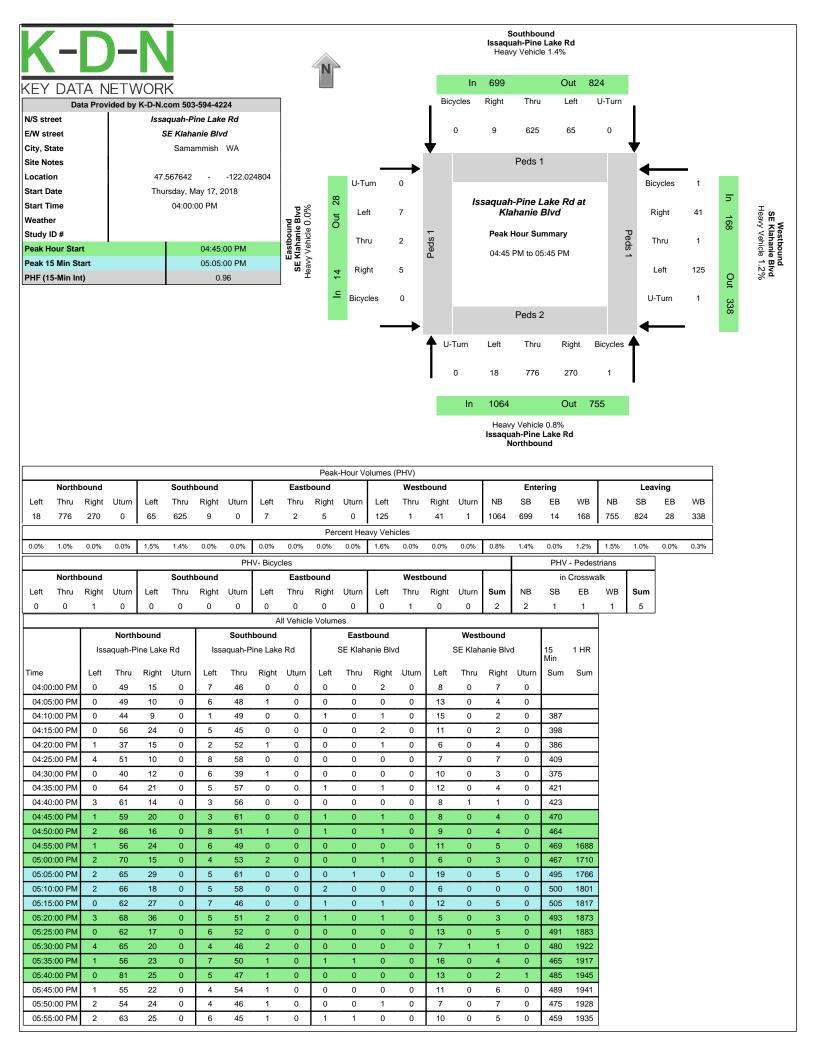


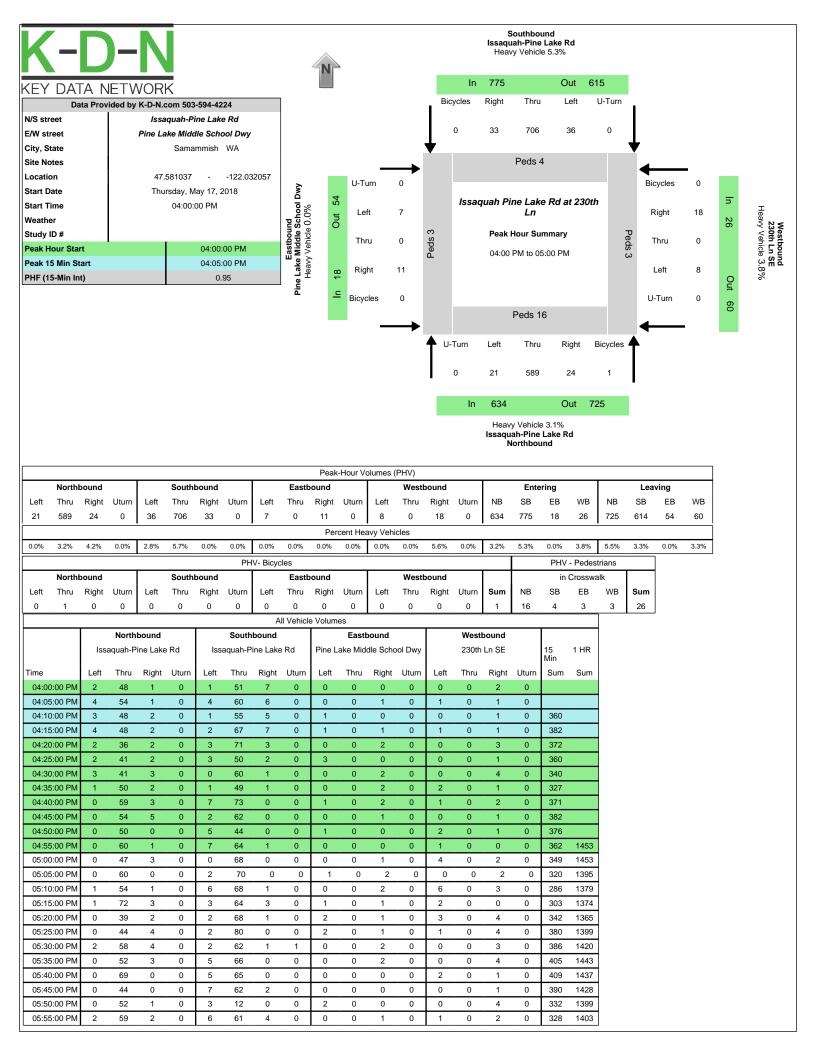


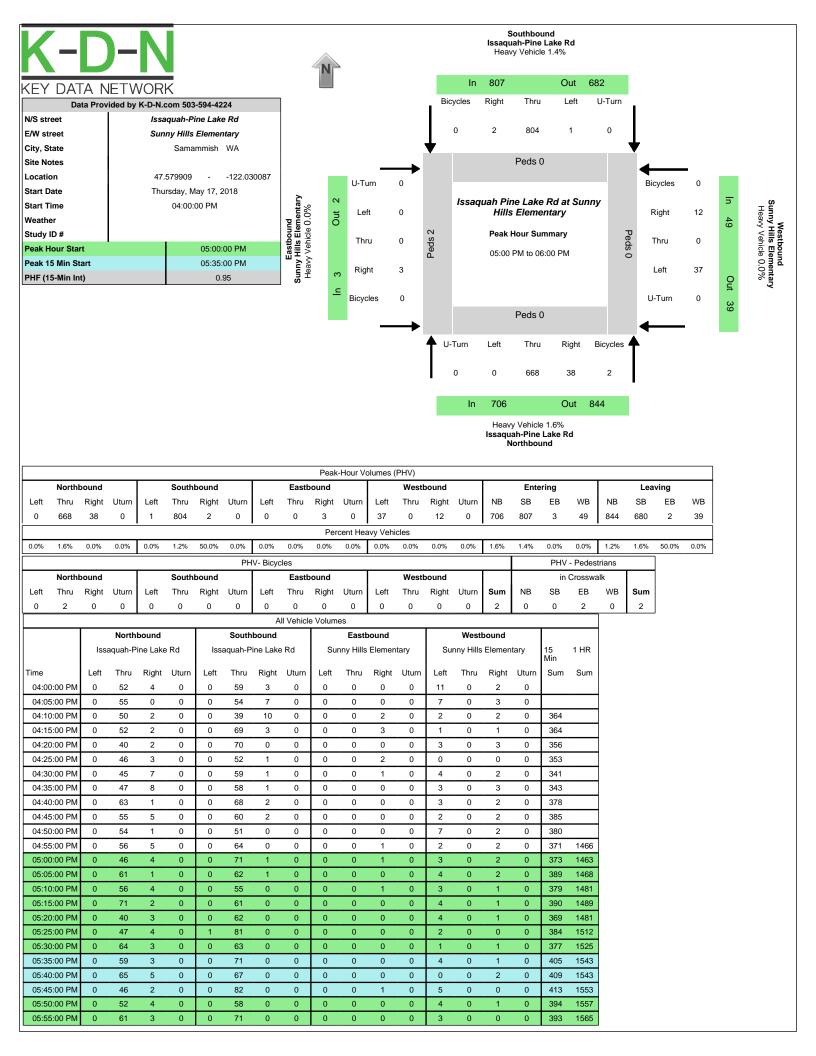


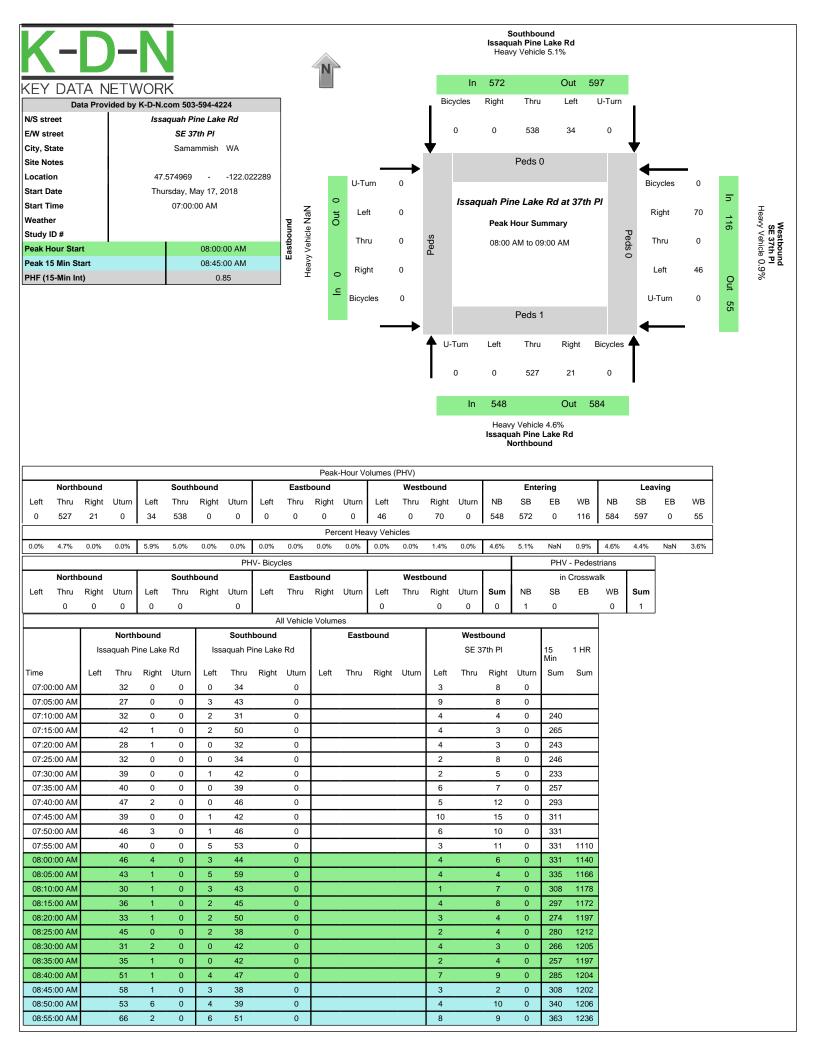


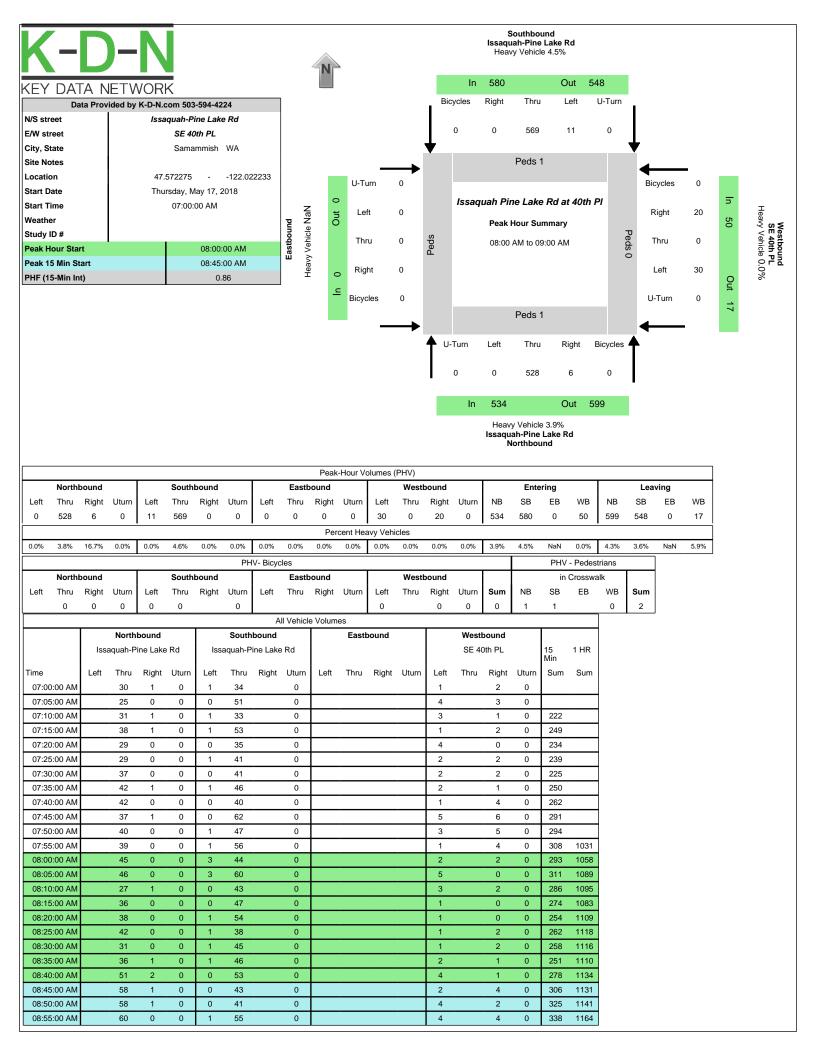


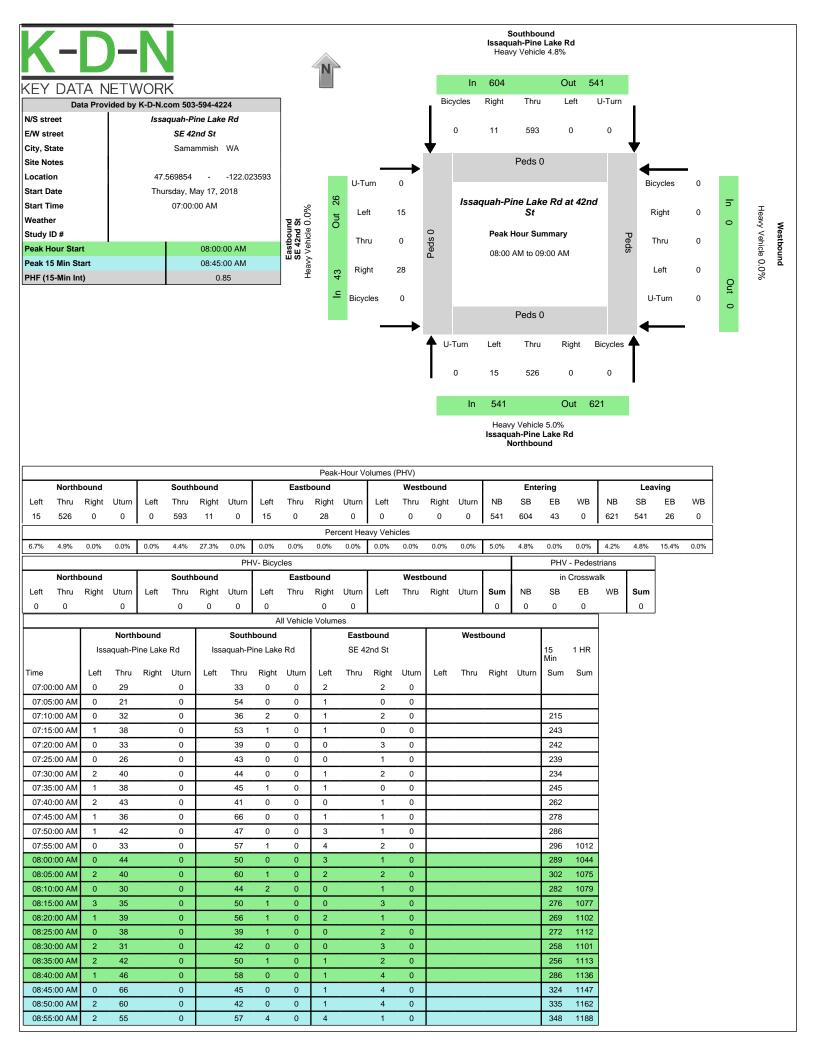


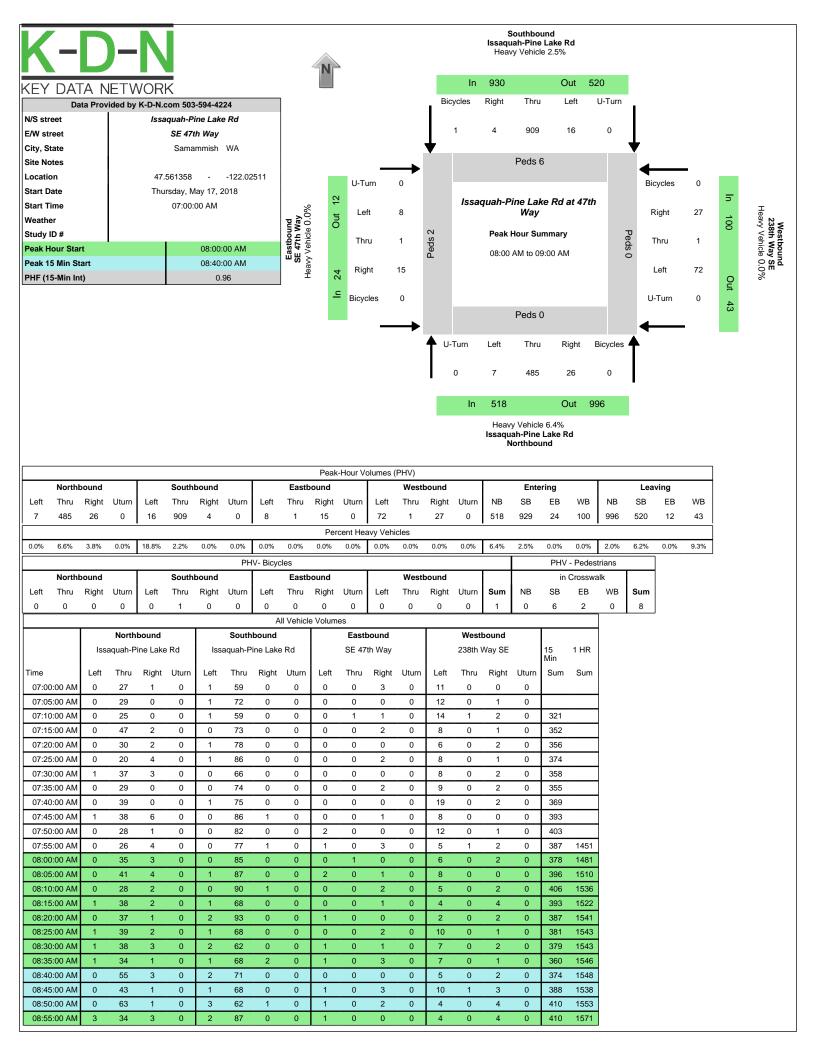


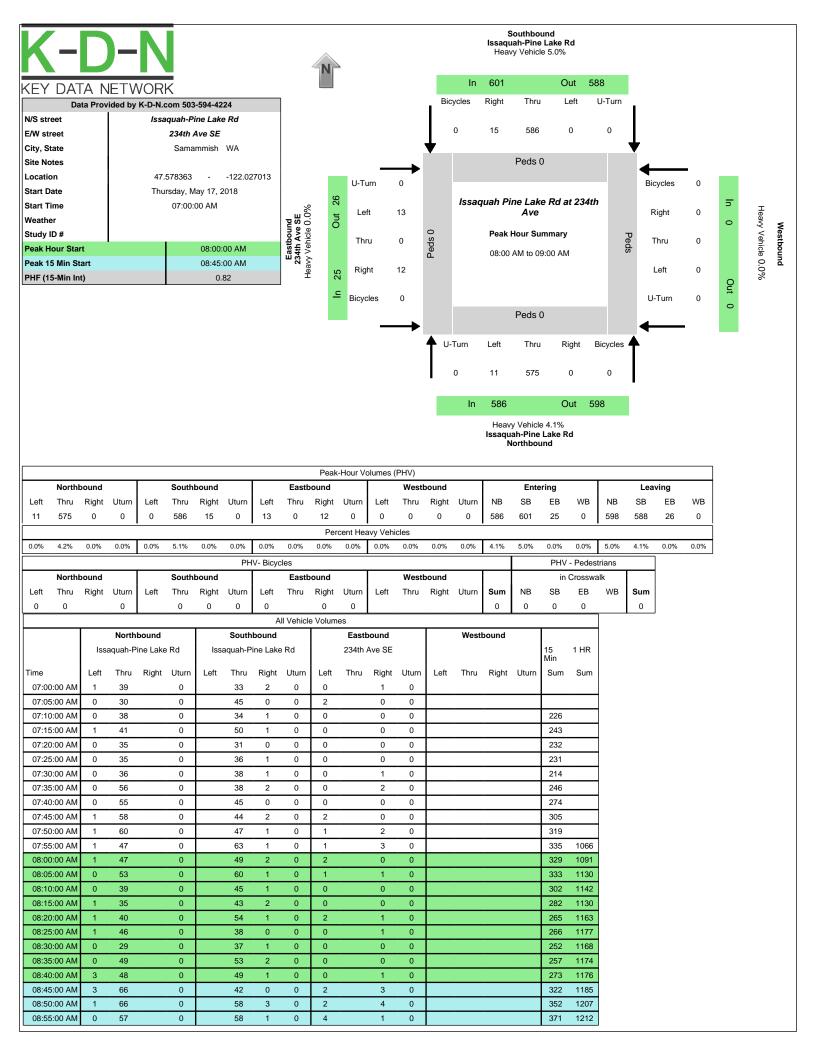


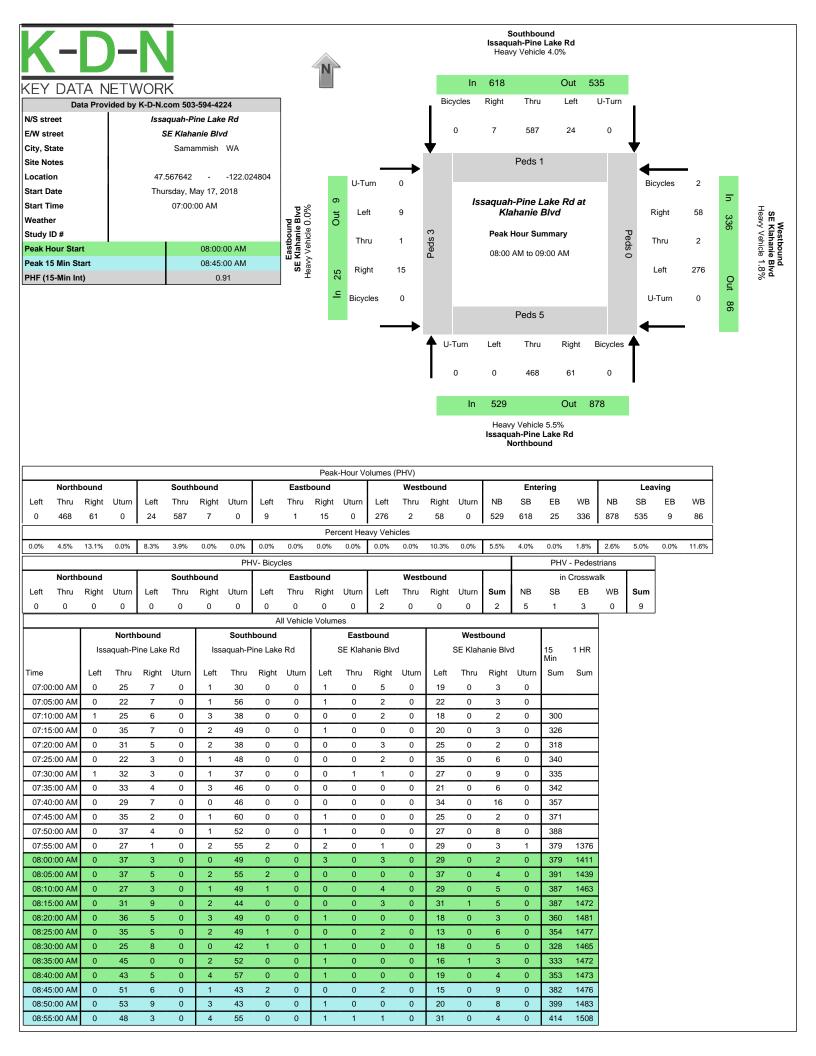


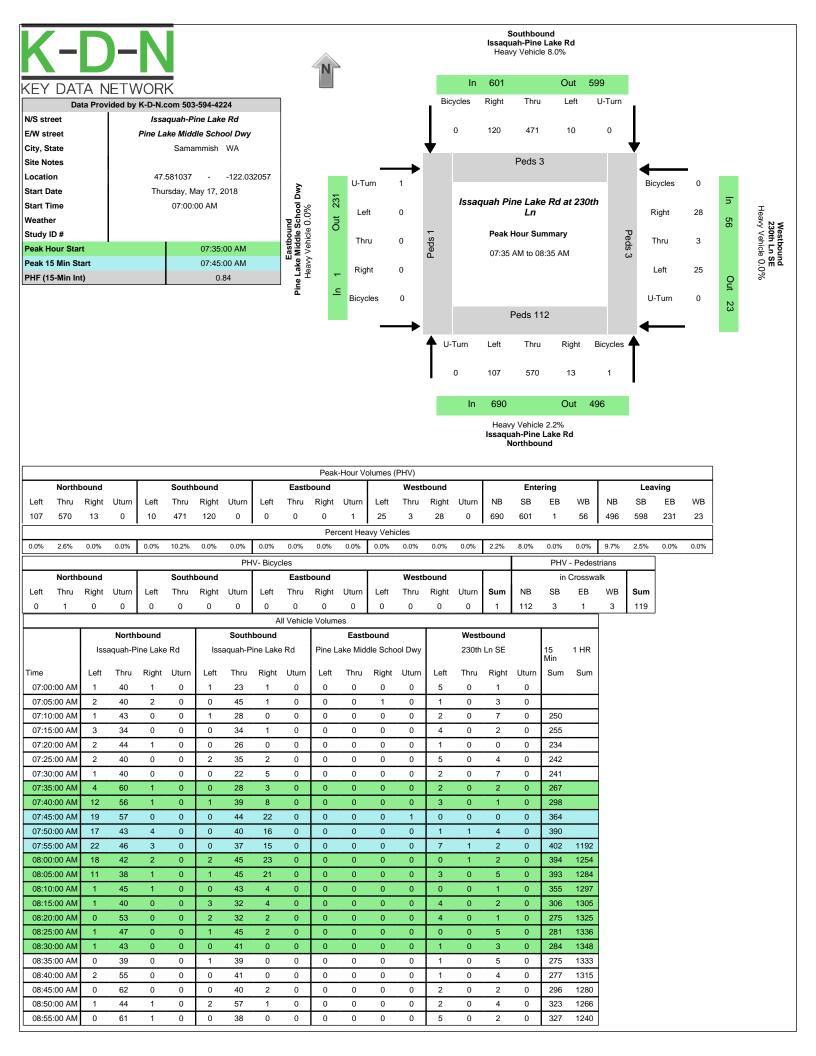


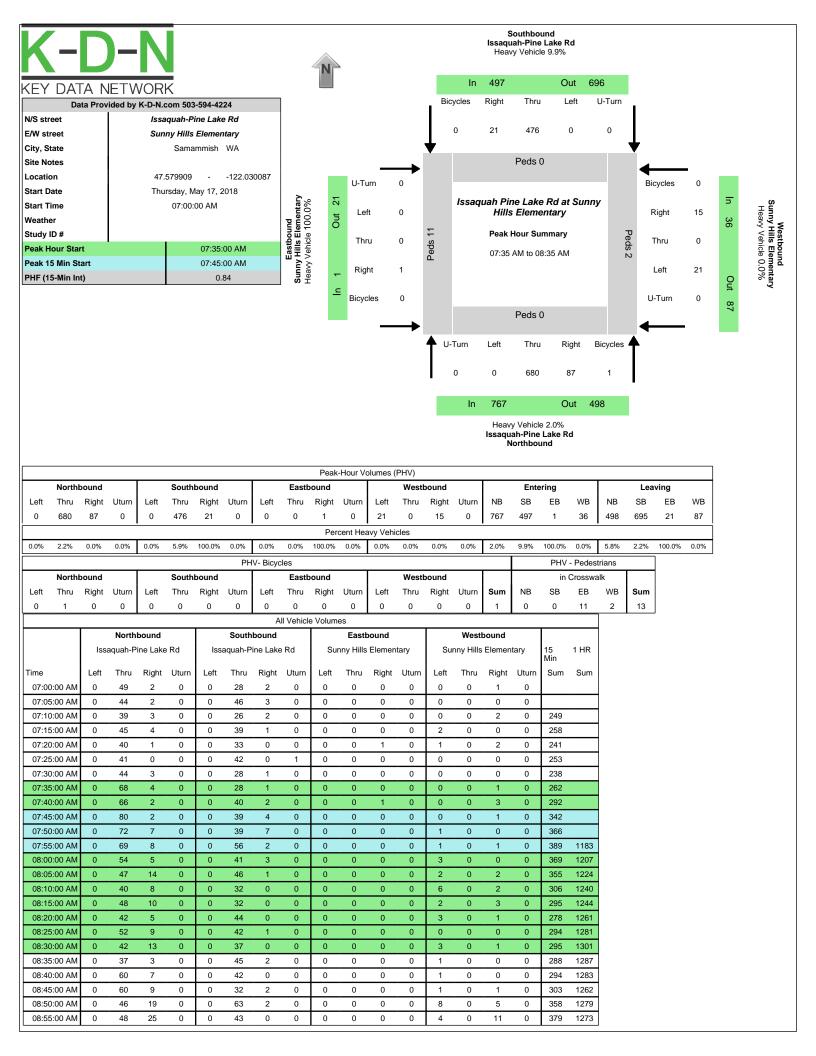












Study Name Issaquah Pine Lake Rd at 32nd Way Start Date 05/17/2018 Start Time 7:00 AM Site Code

Location 47.578935

-122.028155

		Northbound Northb			\$	Southbound Southb				Eastbound Eastb				Westbound Westb		
Start Time	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn
7:00 AM	0	99	10	0	15	77	0	2	0	0	0	0	36	2	35	0
7:15 AM	0	94	18	0	27	81	1	0	3	0	1	0	37	1	37	0
7:30 AM	0	123	21	0	15	75	0	1	5	0	1	0	51	2	54	0
7:45 AM	1	145	15	1	24	106	0	2	3	0	1	0	49	0	84	0
8:00 AM	1	126	17	0	23	106	0	4	1	0	0	0	52	0	34	0
8:15 AM	0	110	12	0	19	99	0	6	3	0	2	0	36	0	47	0
8:30 AM	0	109	11	0	18	106	1	6	2	0	1	0	37	0	47	0
8:45 AM	0	155	33	4	28	111	1	12	2	0	2	0	47	0	55	0
4:00 PM	0	121	43	0	38	133	2		2	0	1	0	30	0	39	0
4:15 PM	2	95	33	1	48	145	0	0	1	0	2	0	29	0	44	0
4:30 PM	0	118	42	0	42	147	1	2	3	0	1	0	38	1	50	0
4:45 PM	1	141	53	4	53	144	2	0	0	0	1	0	30	0	31	0
5:00 PM	2	126	63	3	50	155	0	2	1	0	3	0	38	0	43	0
5:15 PM	2	129	60	0	56	144	0	1	1	0	0	0	42	0	34	0
5:30 PM	0	144	57	1	65	130	3	1	3	0	0	0	37	0	50	0
5:45 PM	3	127	63	3	66	148	5	3	1	0	1	0	31	0	35	0
8:00 AM	0%	0%	6%	0%	4%	3%	0%	0%	0%	0%	0%	0%	4%	0%	0%	0%
8:15 AM	0%	5%	0%	0%	11%	6%	0%	0%	0%	0%	0%	0%	6%	0%	0%	0%
8:30 AM	0%	6%	9%	0%	17%	5%	0%	0%	0%	0%	0%	0%	5%	0%	4%	0%
8:45 AM	0%	3%	12%	0%	0%	5%	0%	0%	0%	0%	0%	0%	2%	0%	2%	0%
			4%				5%				0%				3%	
5.00 PM	00/	20/	00/	20/	00/	00/	00/	00/	00/	00/	00/	00/	20/	20/	20/	00/
5:00 PM	0%	2%	0%	0%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
5:15 PM	0%	2%	2%	0%	2%	1%	0%	0%	0%	0%	0%	0%	5%	0%	0%	0%
5:30 PM	0%	1%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	2%	0%
5:45 PM	0%	3%	0% 1%	0%	2%	2%	0% 1%	0%	0%	0%	0% 0%	0%	0%	0%	0% 1%	0%

Key Data Network K-D-N.com

Issaquah-Pine Lake Rd North of 37th PI Start: 5-15-18 7:00PM

Start	16-May-18		SB		NB	<u> </u>	mbined	17-May	.,	SB		NB	Con	nbined
Time	Wed	A.M.		I. A.M				Thu	y A.M		. A.M			P.M.
12:00	vvcu	11	87	10	96	21	183	IIIu	10	104	. A.IVI	95	17	199
12:15		4	99	11	98	15	197		7	91	10	107	17	198
12:30		6	81	4	82	10	163		5	95	2	100	7	195
12:45		4	102	4	87	8	189		1	98	3	100	4	198
01:00		3	111	3	108	6	219		0	97	3	87	3	184
01:15		2	111	1	107	3	218		3	103	2	82	5	185
01:30		1	133	2	103	3	236		3	96	2	80	5	176
01:45		0	90	6	108	6	198		2	107	1	100	3	207
02:00		5	117	1	94	6	211		2	110	2	89	4	199
02:15		2	89	2	88	4	177		2	95	5	98	7	193
02:30		1	108	4	108	5	216		3	102	3	151	6	253
02:45		2	104	4	138	6	242		2	139	2	140	4	279
03:00		4	120	4	119	8	239		3	184	1	129	4	313
03:15		2	154	0	129	2	283		2	163	2	136	4	299
03:30		1	183	2	135	3	318		2	152	6	131	8	283
03:45		5	172	2	144	7	316		7	180	4	150	11	330
04:00		2	198	6	125	8	323		7	164	7	143	14	307
04:15		3	163	0	131	3	294		5	173	6	143	11	316
04:30		6	158	4	158	10	316		14	180	6	165	20	345
04:45		16	181	8	177	24	358		12	169	6	178	18	347
05:00		15	168	14	167	29	335		15	190	13	191	28	381
05:15		14	189	13	209	27	398		8	187	18	194	26	381
05:30		32	146	28	202	60	348		26	171	21	195	47	366
05:45		37	164	33	211	70	375		30	154	30	190	60	344
06:00		39	171	38	188	77	359		36	156	38	153	74	309
06:15		73	132	58	175	131	307		70	147	55	155	125	302
06:30		92	147	74	208	166	355		89	172	61	166	150	338
06:45		96	134	90	159	186	293		95	129	101	138	196	267
07:00		108	119	98	114	206	233		109	114	105	132	214	246
07:15 07:30		116	133	127	116 107	243 230	249 213		129 118	102	113 148	105	242 266	207
		124	106	106			204			108		93 91		201
07:45 08:00		126 103	98 90	127 111	106 88	253 214	178		141 158	125 151	162 135	102	303 293	216 253
08:15		113	90	119	92	232	189		137	80	133 129	75	266	155
08:30		154	81	121	73	232 275	154		124	82	129	70	253	152
08:45		124	83	179	64	303	147		138	85	193	68	331	153
09:00		149	70	134	79	283	147		162	79	141	81	303	160
09:00		148	70	103	56	251	126		157	65	105	68	262	133
09:13		146	57	103	50	254	107		112	40	112	56	224	96
09:45		90	41	154	42	244	83		87	34	82	41	169	75
10:00		134	35	91	28	225	63		79	21	92	38	171	59
10:15		128	13	75	42	203	55		76	15	74	42	150	57
10:30		96	18	79	29	175	47		111	22	73	29	184	51
10:45		89	12	90	17	179	29		83	13	74	17	157	30
11:00		119	15	79	19	198	34		106	10	71	14	177	24
11:15		99	10	75	12	174	22		73	14	75	18	148	32
11:30		91	4	109	16	200	20		90	5	83	11	173	16
11:45		115	15	115	8	230	23		95	8	94	15	189	23
Total		2850	4979	2626	5012	5476	9991		2746	5081	2607	4952	5353	10033
Day Tota			329		638		467			827		559	153	
% Total	18	8.4%	32.2%	17.0%	32.4%				17.8%	33.0%	16.9%	32.2%		
Peak	- 0	08:30	03:30	08:15	05:15	08:30	05:15	-	08:30	04:30	08:15	05:00	08:15	04:45
Vol.	-	575	716	553	810	1112	1480	-	581	726	592	770	1153	1475
P.H.F.	C	0.933	0.904	0.772	0.960	0.917	0.930		0.897	0.955	0.767	0.987	0.871	0.968
				-						2.300				2.000

ADT

ADT 15,426

AADT 15,426

Key Data Network K-D-N.com

Issaquah-Pine Lake Rd North of Klahanie Start: 5-15-18 7:00PM

Start	16-May-18		NB		SB	Co	mbined	17-Ma	y	NB		SB	Cor	nbined
Time	Wed	A.M.	P.M	. A.M.	P.M.	A.M.	P.M.	Thu	A.M.	P.M	. A.M	l. P.M.	A.M.	P.M
12:00		17	92	12	93	29	185		6	94	11	95	17	18
12:15		12	104	4	82	16	186		13	119	4	95	17	2
12:30		5	87	6	94	11	181		2	95	4	103	6	1
12:45		4	88	3	101	7	189		6	136	1	95	7	2
01:00		3	118	2	112	5	230		3	98	1	115	4	2
01:15		1	106	2	102	3	208		3	88	2	122	5	2
01:30		3	99	1	138	4	237		2	82	3	95	5	1
01:45		5	112	0	99	5	211		2	99	2	99	4	1
02:00		3	99	5	119	8	218		5	100	3	107	8	2
02:15		2	92	2	101	4	193		5	114	2	93	7	2
02:30		4	115	1	118	5	233		2	132	3	104	5	2
02:45		4	128	2	115	6	243		2	124	3	139	5	2
03:00		5	113	3	111	8	224		1	138	3	147	4	2
03:00		0	130	1	133	1	263		2	133	2	165	4	2
03:30			140	_	178		318		7	144		161	9	3
		1		4		5					2			
03:45		3	139	4	168	7	307		3	159	7	191	10	3
04:00		6	135	4	191	10	326		6	150	7	153	13	3
04:15		0	147	6	170	6	317		6	142	7	172	13	3
04:30		5	171	10	155	15	326		5	173	13	168	18	3
04:45		8	170	17	164	25	334		6	185	18	172	24	;
05:00		8	172	17	155	25	327		12	207	17	181	29	
05:15		12	218	22	169	34	387		18	207	17	165	35	3
05:30		27	207	38	148	65	355		19	196	28	159	47	
05:45		37	224	45	155	82	379		31	192	39	162	70	3
06:00		34	191	45	152	79	343		35	165	43	149	78	3
06:15		49	168	84	115	133	283		46	164	83	134	129	2
06:30		71	207	113	131	184	338		59	177	106	172	165	3
06:45		93	165	125	128	218	293		91	152	112	123	203	2
07:00		84	114	129	115	213	229		83	128	128	116	211	2
07:15		112	136	133	113	245	249		95	115	139	111	234	2
07:30		91	121	133	104	224	225		126	92	131	95	257	7
07:45		109	93	152	86	261	179		106	96	171	101	277	
08:00		98	102	121	83	219	185		112	114	159	149	271	2
08:15		108	107	137	93	245	200		121	75	141	75	262	
08:30		113	82	146	79	259			132	81	160			
							161					78	292	•
08:45		155	69	153	74	308	143		169	80	154	66	323	•
09:00		108	101	163	67	271	168		120	91	180	75	300	
09:15		95	61	154	64	249	125		108	76	176	61	284	
09:30		99	63	133	59	232	122		107	70	134	38	241	
09:45		123	49	103	34	226	83		73	45	112	28	185	
10:00		82	35	122	33	204	68		78	42	85	18	163	
10:15		70	45	129	15	199	60		78	42	81	13	159	
10:30		78	38	110	19	188	57		69	34	117	15	186	
10:45		92	22	81	10	173	32		79	18	87	14	166	
11:00		84	21	119	13	203	34		73	17	113	10	186	
11:15		71	16	100	8	171	24		70	17	83	13	153	
11:30		91	17	119	4	210	21		81	15	101	8	182	
11:45		104	10	131	16	235	26		109	14	94	5	203	
Total		2389	5239	3146	4786	5535	10025		2387	5227	3089	4925	5476	10
Day Total			528		932		560			614		014	156	
% Total		5.4%	33.7%	20.2%	30.8%				15.3%	33.4%	19.8%	31.5%		
Peak	- C	8:15	05:15	08:30	03:30	08:30	05:15	-	08:15	05:00	08:30	04:15	08:30	04
Vol.	-	484	840	616	707	1087	1464	-	542	802	670	693	1199	14
P.H.F.	0	.781	0.938	0.945	0.925	0.882	0.946		0.802	0.969	0.931	0.957	0.928	0.9



APPENDIX B:

Signal Warrant

Traffic Signal Warrant Summary Worksheet

The Worksheet(s) attached are provided as an attachment to the Engineering Investigation Study for:

Intersection: Issaquah Pine Lake Road @ SE 37th Pl

County: King

City: Sammamish

Major Street: Issaquah Pine Lake Road Minor Street: SE 37th Pl

Critical Approach Speed: 45 mph Critical Approach Speed: 25 mph

Lanes: 1 lane Lanes: 1 lane

% Right Turns Included
From North (SB) 100%
From East (WB) 100%
From South (NB) 100%

In built-up area of isolated community of < 10,000 population? No
Total number of approaches at intersection? 3

If it is a "T" intersection, inflate minor threshold to 150%? No
Manually set volume level? No

From West (EB) 100%

Analysis based on **EXISTING** volume data.

Date	Day of the Week		Time (HH	:MM)		
Date	Day of the week	From	AM / PM	То	AM / PM	
18-May-18 Thursday		7:00	AM	6:00	PM	

^{*}No weekend data was collected

Warrant Evaluation Summary	Warrant Met:
Warrant 1: Eight - Hour Vehicular Volume	N/A
Condition A: Minimum Vehicular Volume	
Condition B: Interruption of Continuous Traffic	
Condition C: Combination: 80% of A and B	
Warrant 2: Four-Hour Volume	Yes
Warrant 3: Peak Hour Volume	Yes
Warrant 4: Pedestrian Volume	No
Criterion A: Four-Hour	No
Criterion B: Peak-Hour	No
Warrant 5: School Crossing	No
Warrant 6: Coordinated Signal System	N/A
Warrant 7: Crash Experience	N/A
Warrant 8: Roadway Network	No
Warrant 9: Intersection Near a Grade Crossing	N/A

Warrant Analysis Conducted By:

Name: Josh Anderson, PE, PTOE

Agency: David Evans and Associates, Inc.

Date: 7/30/2018

Warrant 1: Eight - Hour Vehicular Volume

70%

Warrant Evaluated? No

Condition A:						
Min. Veh. Volume						
70%	56%					
350	280					
105	84					
2	2					
	70% 350					

Satisfied?

Condition B:							
Interruption of Continuous Traffic							
Volume Level	70%	56%					
Major Rd. Req	525	420					
Minor Rd. Req	53	42					
Number of Hours	4	4					

Satisfied?

Condition C:
Combination of A & B at 56%
Satisfied?

Warrant	Satisfied?	N/A	

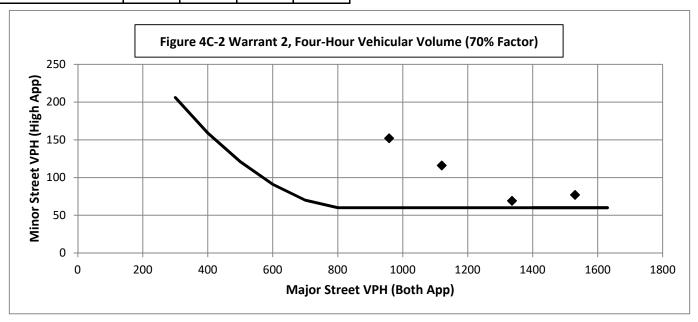
Manually Set To:

6:00 AM		Enter	Start Time (Military	Time) (HH:MM)	
Time Period	From	То	Major Road: Both App. (VPH)	Minor Road: High App. (VPH)	Total
1	6:00	7:00	0	0	0
2	7:00	8:00	958	152	1110
3	8:00	9:00	1120	116	1236
4	9:00	10:00	0	0	0
5	10:00	11:00	0	0	0
6	11:00	12:00	0	0	0
7	12:00	13:00	0	0	0
8	13:00	14:00	0	0	0
9	14:00	15:00	0	0	0
10	15:00	16:00	0	0	0
11	16:00	17:00	1336	69	1405
12	17:00	18:00	1530	77	1607
13	18:00	19:00	0	0	0
14	19:00	20:00	0	0	0
15	20:00	21:00	0	0	0
16	21:00	22:00	0	0	0

Warrant 2: Four-Hour Volume

70%

					Warrant Evaluated? Yes
Hour Start	17:00	16:00	8:00	7:00	Warrant Satisfied? Yes
Major Road Vol.	1530	1336	1120	958	Manually Set To:
Minor Road Vol.	77	69	116	152	



Warrant 3: Peak Hour Volume

70%

Warrant Evaluated? Yes

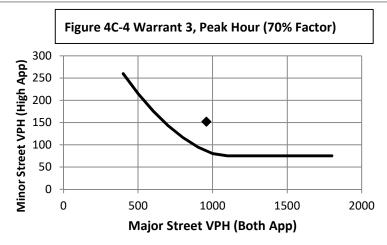
Condition justifying use of warrant:

Criteria	Met?	
Delay on Minor Approach	4	No
Volume on Minor Approach	100	Yes
Total Entering Volume (veh/h)	650	163

Peak Hour	Major Road Vol.	Minor Road Vol.
reak noui	(Both App.)	(High App.)
7:00	958	152
7:00	958	152

Warrant Satisfied? Yes

Manually Set To:



Warrant 4: Pedestrian Volume

70%

Warrant Evaluated? Yes

No

No

Criterion A: Four Hour

Hour	Pedestrian	Major Road
(Start)	Volume	Vol.
7:00	6	958
8:00	4	1120
16:00	2	1336
17:00	2	1530

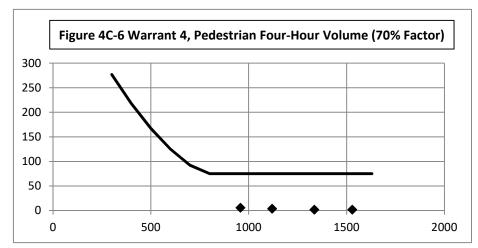
Manually Set Major Rd Vol?

Avg. walk speed less than 3.5 ft/s?

Criterion A Satisfied? No

Warrant Satisfied? No

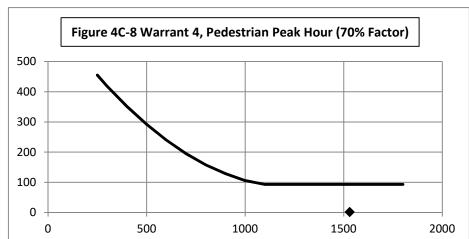
Manually Set To:



Criterion B: Peak Hour

Peak Hour	Pedestrian	Major Road
Peak Hour	Vol.	Vol.
17:00	2	1530

Criterion B Satisfied? No



Warrant 5: School Crossing

70%

Warrant Evaluated? Yes

Warrant Satisfied? No

Manually Set To:

Crit	reria	Fulfilled?
	1 There are a MINIMUM of 20 school children during the highest crossing hour.	
2	There are fewer adequate gaps in the major road traffic stream during the period when the school children are using the crossing than the number of minutes in the same period.	No
3	The nearest traffic signal along the major road is located more than 300 ft away. Or, the nearest traffic signal is within 300 ft but the proposed traffic signal will not restrict the progressive movement of traffic.	Yes

Warrant 6: Coordinated Signal System

70%

Warrant Evaluated? Yes

Warrant Evaluated? No

Appears as a major route on an official plan

Warrant Satisfied? N/A

Warrant Satisfied? N/A

Manually Set To:

Manually Set To:

Criteria			
1 Signal spacing > 1000 ft			
On a one-way road or a road that has traffic predominantly in one direction, the adjacent signals are so far apart that they do not provide the necessary degree of vehicle platooning.			
	On a two-way road, adjacent signals do not provide the necessary degree of platooning and the proposed and the adjacent signals will collectively provide a progressive operation.	No	

Warrant 7: Crash Experience

70%

		raniant bathonical 14,	,	,	
Crite	eria			Met?	Fulfilled?
1	Adequate trial of other remedial measures has failed to redu	ce crash frequency.			No
	Measures Tried: None				NO
2	Five or more reported crashes, of types susceptible to correc	ction by signal, have	# of crashes per 12	months	No
	occurred within a 12 month period.		3		NO
	Warrant 1, Condition A (80%)			No	
3	Warrant 1, Condition B (80%)			No	No
3	Warrant 4, Criterion A (80%)			No	NO
	Warrant 4, Criterion B (80%)			No	

Warrant 8: Roadway Network

70%

Yes

	Warrant Evaluated? Yes	Warran	t Satisfied?	No	Manua	lly Set To:	
Crit	eria					Met?	Fulfilled?
1	Total entering volume of at least 1,000 veh/h during t	ypical weekda	y peak hour		1110	Yes	No
	Five-year projected volumes that satisfy one or more of Warrants 1, 2, or 3.					No	INO
	Total entering vol. of at least 1,000 veh/h for each of	any 5 hrs of no	n-normal bu	siness day	(Sat. or Sun.))	
2	Hour	0:00	0:00	0:00	0:00	0:00	No
	Volume	9 0	0	0	0	0	
Cha	racteristics of Major Routes - Select yes if all intersect	ing routes hav	e characteri	stic			Fulfilled?
1	Part of the road or highway system that serves as the	principal road	way network	for throug	h traffic flow	/	Yes
2	Rural or suburban highway outside of, entering, or tra	versing a city					No

Warrant 9: Intersection Near a Grade Crossing

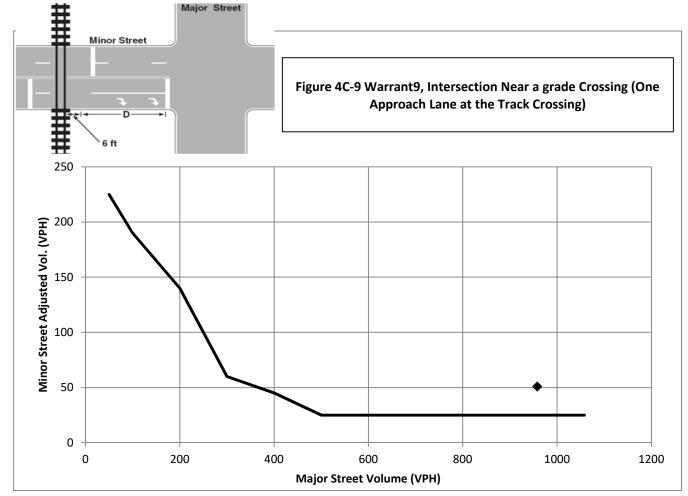
70%

Warrant Evaluated? No

Warrant Satisfied? N/A

Manually Set To:

Adjustment Factors				M	lanually Set	Peak Hour?	
Rail Traffic	% High Occupancy	% Tractor-Trailer Trucks	2	Peak Hour	Major	Minor Road	Adjusted
per Day	Buses on Minor Road	on Minor Road	D	Peak Hour	Road Vol.	Vol.	Minor Vol.
1	0	0% to 2.5%	660	7:00	958	152	50.92



Conclusions/Comments:

This analysis was completed using the following assumptions:

- -The major-street speed is in excess of 40 mph and the intersection is in an isolated community with a population of less than 10,000 (70% warrants)
- -2016 and 2017 crash data not available
- -No significant imminent developments planned on study area roadways

NOTE - Warrants would not be met under the following conditions:

- -100% warrants evaluated
- -Minor street threshold inflated 150% due to "T" intersection configuration

Updated: 2/18/2016

Traffic Signal Warrant Summary Worksheet

The Worksheet(s) attached are provided as an attachment to the Engineering Investigation Study for:

Intersection: Issaquah Pine Lake Road @ SE 37th Pl

County: King

City: Sammamish

Major Street: Issaquah Pine Lake Road Minor Street: SE 37th Pl

Critical Approach Speed: 45 mph Critical Approach Speed: 25 mph

Lanes: 1 lane Lanes: 1 lane

% Right Turns Included
From North (SB) 100%
From East (WB) 100%
From South (NB) 100%
From West (EB) 100%
In built-up area of isolated community of < 10,000 population? No
Total number of approaches at intersection? 3
If it is a "T" intersection, inflate minor threshold to 150%? No
Manually set volume level? No
From West (EB) 100%

Analysis based on **PROJECTED** volume data.

Forecast Year	Within 5 Years of		Time (HH	I:MM)	
Forecast real	Construction?	From	AM / PM	То	AM / PM
18-May-35	No	7:00	AM	6:00	PM

^{*}No weekend data was collected

Warrant Evaluation Summary	Warrant Met:
Warrant 1: Eight - Hour Vehicular Volume	N/A
Condition A: Minimum Vehicular Volume	
Condition B: Interruption of Continuous Traffic	
Condition C: Combination: 80% of A and B	
Warrant 2: Four-Hour Volume	Yes
Warrant 3: Peak Hour Volume	Yes
Warrant 4: Pedestrian Volume	No
Criterion A: Four-Hour	No
Criterion B: Peak-Hour	No
Warrant 5: School Crossing	No
Warrant 6: Coordinated Signal System	N/A
Warrant 7: Crash Experience	N/A
Warrant 8: Roadway Network	No
Warrant 9: Intersection Near a Grade Crossing	N/A

Warrant Analysis Conducted By:

Name: Josh Anderson, PE, PTOE

Agency: David Evans and Associates, Inc.

Date: 7/30/2018

Warrant 1: Eight - Hour Vehicular Volume

70%

Warrant	Evalua	ated?	No
---------	--------	-------	----

Condition A:				
Min. Veh. Volume				
Volume Level 70% 56%				
Major Rd. Req	350	280		
Minor Rd. Req	105	84		
Number of Hours 2 2				

Satisfied?

Condition B:						
Interruption of Continuous Traffic						
Volume Level 70% 56%						
Major Rd. Req	525	420				
Minor Rd. Req	53	42				
Number of Hours	4	4				

Satisfied?

Condition C:
Combination of A & B at 56%
Satisfied?

Warrant Satisfied? N/A

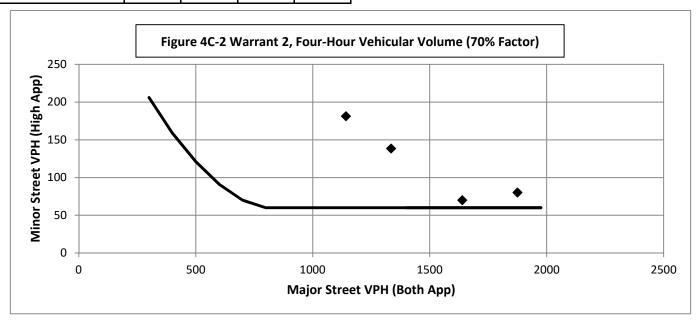
Manually Set To:

6:00 AM Enter			Start Time (Military	Time) (HH:MM)	
Time Period	From	То	Major Road: Both App. (VPH)	Minor Road: High App. (VPH)	Total
1	6:00	7:00	0	0	0
2	7:00	8:00	1142	181	1322.99
3	8:00	9:00	1335	138	1473.16
4	9:00	10:00	0	0	0
5	10:00	11:00	0	0	0
6	11:00	12:00	0	0	0
7	12:00	13:00	0	0	0
8	13:00	14:00	0	0	0
9	14:00	15:00	0	0	0
10	15:00	16:00	0	0	0
11	16:00	17:00	1639	70	1709.26
12	17:00	18:00	1875	80	1955
13	18:00	19:00	0	0	0
14	19:00	20:00	0	0	0
15	20:00	21:00	0	0	0
16	21:00	22:00	0	0	0

Warrant 2: Four-Hour Volume

70%

Hour Start	17:00	16:00	8:00	7:00
Major Road Vol.	1875	1639.31	1334.91	1141.82
Minor Road Vol.	80	69.944	138.258	181.166



Warrant 3: Peak Hour Volume

70%

Warrant Evaluated? Yes

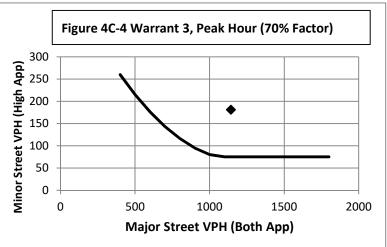
Condition justifying use of warrant:

Criteria	Met?	
Delay on Minor Approach	4	No
Volume on Minor Approach	100	Yes
Total Entering Volume (veh/h)	650	163

ſ	Peak Hour	Major Road Vol.	Minor Road Vol.
	Peak Houl	(Both App.)	(High App.)
	7:00	1142	181
	7:00	1142	181

Warrant Satisfied? Yes

Manually Set To:



Warrant 4: Pedestrian Volume

70%

Warrant Evaluated? Yes

No

No

Criterion A: Four Hour

Hour	Pedestrian	Major Road		
(Start)	Volume	Vol.		
7:00	6	1141.8206		
8:00	4	1334.905		
16:00	2	1639.3124		
17:00	2	1875		

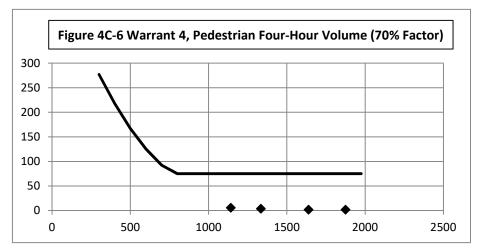
Manually Set Major Rd Vol?

Avg. walk speed less than 3.5 ft/s?

Criterion A Satisfied? No

Warrant Satisfied? No

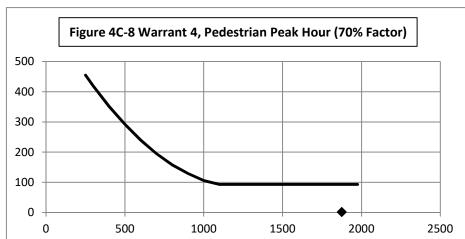
Manually Set To:



Criterion B: Peak Hour

Peak Hour	Pedestrian	Major Road
Peak Hour	Vol.	Vol.
17:00	2	1875

Criterion B Satisfied? No



Warrant 5: School Crossing

70%

Warrant Evaluated? Yes

Warrant Satisfied? No

Manually Set To:

Crit	reria	Fulfilled?
	There are a MINIMUM of 20 school children during the highest crossing hour.	No
2	There are fewer adequate gaps in the major road traffic stream during the period when the school children are using the crossing than the number of minutes in the same period.	No
3	The nearest traffic signal along the major road is located more than 300 ft away. Or, the nearest traffic signal is within 300 ft but the proposed traffic signal will not restrict the progressive movement of traffic.	Yes

Warrant 6: Coordinated Signal System

70%

Warrant Evaluated? Yes

Appears as a major route on an official plan

Warrant Satisfied? N/A

Manually Set To:

Crit	teria	Fulfilled?
1	Signal spacing > 1000 ft	No
2	On a one-way road or a road that has traffic predominantly in one direction, the adjacent signals are so far apart that they do not provide the necessary degree of vehicle platooning.	No
3	On a two-way road, adjacent signals do not provide the necessary degree of platooning and the proposed and the adjacent signals will collectively provide a progressive operation.	No

Warrant 7: Crash Experience

70%

	Warrant Evaluated? No	Warrant Satisfied? N	I/A Manuall	y Set To:	
Crite	eria			Met?	Fulfilled?
1	Adequate trial of other remedial measures has failed	to reduce crash frequency.			No
	Measures Tried: None				INO
2	Five or more reported crashes, of types susceptible t	to correction by signal, have	# of crashes per 12 n	nonths	No
	occurred within a 12 month period.		3		INO
	Warrant 1, Condition A (80%)			No	
3	Warrant 1, Condition B (80%)			No	No
3	Warrant 4, Criterion A (80%)			No	INO
	Warrant 4, Criterion B (80%)			No	

Warrant 8: Roadway Network

70%

Yes

	Warrant Evaluated? Yes	Warrant	Satisfied?	No	Manua	lly Set To:	
Crit	eria					Met?	Fulfilled?
1	Total entering volume of at least 1,000 veh/h during typica	al weekday	peak hour		1323	Yes	No
	Five-year projected volumes that satisfy one or more of W	arrants 1,	2, or 3.			No	T NO
	Total entering vol. of at least 1,000 veh/h for each of any 5	hrs of nor	n-normal bu	siness day ((Sat. or Sun.)		
2	Hour	0:00	0:00	0:00	0:00	0:00	No
	Volume	0	0	0	0	0	
Cha	Characteristics of Major Routes - Select yes if all intersecting routes have characteristic						Fulfilled?
1	Part of the road or highway system that serves as the princ	cipal roadv	ay network	for throug	h traffic flov	/	Yes
2	Rural or suburban highway outside of, entering, or travers	ing a city					No

Warrant 9: Intersection Near a Grade Crossing

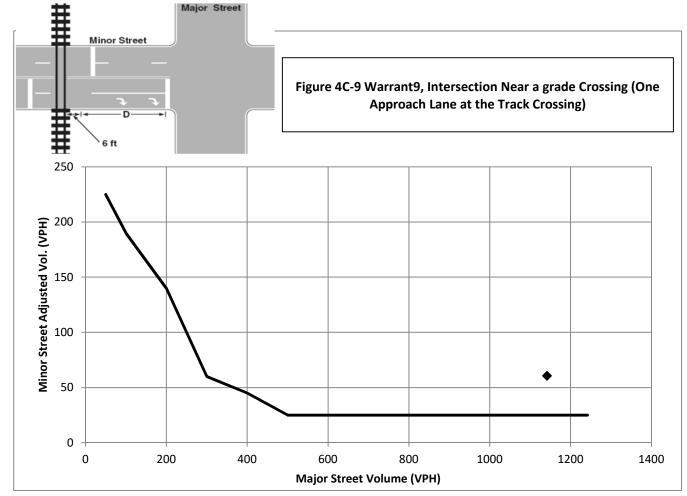
70%

Warrant Evaluated? No

Warrant Satisfied? N/A

Manually Set To:

Adjustment Factors			Manually Set Peak Hour?				
Rail Traffic	% High Occupancy	% Tractor-Trailer Trucks	D	Peak Hour	Major	Minor Road	Adjusted
per Day	Buses on Minor Road	on Minor Road			Road Vol.	Vol.	Minor Vol.
1	0	0% to 2.5%	660	7:00	1142	181	60.635



Conclusions/Comments:

This analysis was completed using the following assumptions:

- -The major-street speed is in excess of 40 mph and the intersection is in an isolated community with a population of less than 10,000 (70% warrants)
- -2016 and 2017 crash data not available
- -No significant imminent developments planned on study area roadways

NOTE - Warrants would not be met under the following conditions:

- -100% warrants evaluated
- -Minor street threshold inflated 150% due to "T" intersection configuration

Updated: 2/18/2016